Plastic Energy Dissipation in Pressure-Dependent Materials

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8 ABSTRACT

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Presented is a thermodynamics-based energy analysis approach for pressure-dependent mate-9 rials. Formulation of plastic free energy and plastic dissipation for non-associated Drucker-Prager 10 plasticity model is derived based on thermodynamics. It is proven that the proposed energy com-11 putation formulation always gives non-negative incremental plastic dissipation, as required by the 12 second law of thermodynamics. Presented methodology is illustrated using numerical simulations 13 of Toyoura sand and Sacramento river sand under different loading conditions. Multi-directional 14 loading and pressure-dependency effects on plastic dissipation are investigated. The continuous, 15 non-negative dissipation of mechanical energy in pressure-dependent frictional materials under 16 complex 3D cyclic loading is properly modeled. 17

18 INTRODUCTION

Energy dissipation analysis has gained popularity in recent studies of response of dynamic/static inelastic systems. Plastic energy dissipation, if correctly modeled, can be used as an effective indicator of material damage. It is important to distinguish and properly model different energy dissipation mechanisms. These include plastic energy dissipation, viscous damping, and algorithmic (or numerical) damping for numerical modeling of inelastic material (Yang et al. 2018b). Plastic energy dissipation is defined as the thermal energy irreversibly transformed from mechan ical energy during a dissipative process. A thermodynamics-based framework is developed and
 illustrated to model the plastic energy dissipation for pressure-dependent inelastic materials.

Although energy dissipation has been used to explain behavior of soil and structural systems, 27 there exists a common misconception about plastic work and plastic energy dissipation, especially 28 in the field of structural and geotechnical engineering. Plastic work is defined as the material work 29 done due to plastic deformation. In the classic elastoplasticity theory, the increment of plastic work 30 over a time step is calculated by multiplying the effective stress tensor with the increment of plastic 31 strain tensor, $dW^{pl} = \sigma_{ij} d\epsilon_{ij}^{pl}$. As pointed out by Collins and Hilder (2003), this misconception 32 is originated from the decades-old view that, in granular materials, all permanent deformation 33 contributes to the frictional slipping between particles, and thus all the plastic work is dissipated 34 (Luong 1986; Okada and Nemat-Nasser 1994). However, a closer examination of these publications 35 reveals that plastic dissipation was not quantitatively measured in any of these studies. In other 36 words, the assumption that plastic work equals to plastic dissipation for granular materials has never 37 been validated. 38

The difference between plastic work and plastic energy dissipation is defined as the plastic free 39 energy, also known as stored plastic work or cold work. Plastic free energy developed in inelastic 40 material during plastic loading has been observed in physical experiments (Farren and Taylor 1925; 41 Taylor and Quinney 1934; Rittel 2000) and discussed in a number of modeling studies (Collins and 42 Houlsby 1997; Dafalias and Popov 1975; Rosakis et al. 2000; Collins and Kelly 2002; Veveakis 43 et al. 2007; Feigenbaum and Dafalias 2007; Yang et al. 2018a). The physical interpretation of plastic 44 free energy was explained in detail through a conceptual example by Yang et al. (2018a). In short, 45 plastic free energy is the part of plastic work that results in the rearrangement of particles, or change 46 in material fabric, rather than the frictional slipping between particles. It is a thermodynamically 47 essential form of energy when a physically discontinuous material is modeled as a continuum. This 48 point will be further pursued in this paper. 49

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In order to model the plastic energy dissipation in a thermodynamically appropriate fashion,

general theories of thermomechanics of inelastic materials were established by Ziegler and Wehrli 51 (1987) and Lubliner (1990). Collins and Houlsby (1997) reiterated the theory and applied it 52 to the modeling of geotechnical materials. Following studies by Collins et al. (Collins 2002; 53 Collins and Kelly 2002; Collins and Hilder 2003; Collins 2003; Collins and Muhunthan 2003) 54 presented detailed procedures for constructing critical state models that are popular for soils using 55 the thermomechanical framework. One important discovery is that Drucker's postulate, which was 56 considered to be an equivalent condition to the second law of thermodynamics, cannot be used to 57 check the thermodynamic validity of constitutive models. Particularly, the original Cam clay model 58 (Schofield and Wroth 1968) was found to violate the second law of thermodynamics (Collins and 59 Kelly 2002). 60

The constitutive models developed in the previously mentioned studies are significant in the 61 sense of their sound thermodynamics basis. The material response, for example the stress and strain 62 parameters, is controlled by a predefined free energy function and a dissipation function. Compared 63 to the classic elastoplasticity models, these thermomechanical models are more complicated, and 64 thus difficult to be implemented and used in engineering designs. An effort to incorporate the 65 thermomechanical formulation into the classic elastoplasticity theory was made by Feigenbaum 66 and Dafalias (Feigenbaum and Dafalias 2007) for von Mises type material models, which is 67 pressure-independent. Yang et al. (Yang et al. 2018a) extended the formulation to finite element 68 method (FEM) for the energy analysis of inelastic solids. As a continued work, this study focuses 69 on the plastic energy dissipation analysis of pressure-dependent inelastic materials. 70

In the following section, the theoretical formulations of thermomechanics and classic elastoplasticity are summarized and discussed. Equations of the plastic free energy and plastic energy dissipation for pressure-dependent material, modeled using Drucker-Prager plasticity, are derived. It is proven that the presented formulation upholds the first and second laws of thermodynamics, for both von Mises and Drucker-Prager plasticity with various hardening rules. Numerical examples on constitutive level and finite element level are used to illustrate the presented energy computation methodology.

78 PLASTIC WORK AND PLASTIC DISSIPATION

Thermodynamics-based energy computation formulation for pressure-dependent material is presented and discussed in this section. Material parameters and internal variables from thermomechanical theory are very different than those from the classic elastoplasticity theory. The main challenge is to develop energy equations, in terms of the parameters from classic elastoplasticity, that are within the thermomechanical framework, so that the energy results follow the principles of thermodynamics.

Thermomechanical Framework

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The local incremental form of the first law of thermodynamics for an isothermal process is given by (Collins and Houlsby 1997; Yang et al. 2018a)

$$\sigma_{ij}d\epsilon_{ij} = d\psi + \phi \tag{1}$$

where σ_{ij} is the effective stress tensor, ϵ_{ij} is the total small strain tensor, ψ is the (Helmholtz) free energy density function, and ϕ is the incremental plastic energy dissipation density function. The sign convention of stress and strain components follows the traditional mechanics of materials, i.e. positive in tension.

The free energy density function is assumed to be decomposed into an elastic part, known as the elastic strain energy density ψ^{el} , and a plastic part, defined as the plastic free energy density ψ^{pl}

$$d\psi = d\psi^{el} + d\psi^{pl} \tag{2}$$

This decomposition naturally rises when the material is assumed to be of the decoupled type (Collins and Houlsby 1997), which means that the strain tensor can also be additively decomposed into elastic and plastic components. Note that this assumption is also used in the classic small deformation elastoplasticity theory. The incremental elastic strain energy density can then be written as

 $d\psi^{el} = \sigma_{ij} d\epsilon^{el}_{ij} \tag{3}$

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¹⁰² The plastic free energy density ψ^{pl} is related to the evolution of material model internal variables, ¹⁰³ and thus further decomposed into parts that correspond to different hardening types (Feigenbaum ¹⁰⁴ and Dafalias 2007). In this study, isotropic and kinematic hardening rules of various types are ¹⁰⁵ considered. The material model internal variables used in this study are scalar parameter *k*, for ¹⁰⁶ isotropic hardening, defined as the size of yield surface in stress space, and back stress tensor α_{ij} , ¹⁰⁷ for kinematic hardening, defined as the center of yield surface in stress space. The incremental ¹⁰⁸ plastic free energy density then becomes

$$d\psi^{pl} = d\psi^{iso}(\sigma_{ij}, d\epsilon^{pl}_{ij}, k) + d\psi^{kin}(\sigma_{ij}, d\epsilon^{pl}_{ij}, \alpha_{ij})$$

$$\tag{4}$$

Next, the plastic energy dissipation density function is expressed in terms of the plastic free
 energy density:

 $\phi = \sigma_{ij} d\epsilon_{ij} - d\psi = \sigma_{ij} d\epsilon_{ij}^{pl} - (d\psi^{iso} + d\psi^{kin})$ (5)

According to the second law of thermodynamics, the incremental plastic dissipation must always be non-negative during any loading increment. When the plastic free energy function is defined, as will be shown in the next sections, the plastic energy dissipation can be calculated from Equation 5.

Review of Energy Computation for von Mises Plasticity

In this section, the equations of plastic free energy and plastic dissipation for pressureindependent von Mises material model (Yang et al. 2018a) are revisited. Note that von Mises plasticity always uses associated plastic flow rule. In other words, the plastic flow direction in the stress space is normal to the yield surface.

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The yield function of von Mises is expressed in the following form

$$f = \sqrt{(s_{ij} - \alpha_{ij})(s_{ij} - \alpha_{ij})} - \sqrt{\frac{2}{3}}k$$
(6)

where $s_{ij} = \sigma_{ij} - (1/3)\sigma_{kk}$ is the deviatoric part of the stress tensor.

Once the material yields, plastic strain starts to develop. The incremental plastic strain tensor

is calculated as:

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$$d\epsilon_{ij}^{pl} = m_{ij}d\lambda \tag{7}$$

where $d\lambda$ is the scalar loading index that equals to the magnitude of incremental plastic strain. Since associated plasticity is used, the normalized plastic flow direction tensor m_{ij} is calculated by taking the gradient of the yield function in the stress space:

$$m_{ij} = \frac{\partial f}{\partial \sigma_{ij}} = \frac{(s_{ij} - \alpha_{ij})}{\sqrt{(s_{mn} - \alpha_{mn})(s_{mn} - \alpha_{mn})}}$$
(8)

Armstrong-Frederick kinematic hardening (Armstrong and Frederick 1966) is a nonlinear strain hardening rule commonly used to model the cyclic inelastic behavior of various types of materials, including metals, alloys, soils, and other structural/geotechnical materials. The evolution of the incremental back stress tensor $d\alpha_{ij}$ is defined

$$d\alpha_{ij} = \left[\frac{2}{3}h_a m_{ij} - c_r \alpha_{ij} \sqrt{\frac{2}{3}m_{rs}m_{rs}}\right] d\lambda$$
(9)

where h_a and c_r are the non-negative hardening constants. When $h_a > 0$ and $c_r = 0$, the nonlinear Armstrong-Frederick hardening becomes linear hardening rule. If $h_a = 0$ and $c_r = 0$, the material model becomes perfectly plastic with no internal variable hardening. Note that isotropic hardening can be defined in a similar form. To avoid repetition, the remaining part of this paper will focus on kinematic hardening.

The plastic free energy density function that was given by Feigenbaum and Dafalias (Feigenbaum
 and Dafalias 2007) and modified by Yang et al. (Yang et al. 2018a) is

$$d\psi^{kin} = \frac{3}{2h_a} \alpha_{ij} d\alpha_{ij} \tag{10}$$

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The plastic dissipation density function can be expressed in terms of the material parameters

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and internal variables. Substituting Equations (10) and (9) into Equation (5):

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$$\phi = \sigma_{ij} d\epsilon_{ij}^{pl} - \frac{3}{2h_a} \alpha_{ij} d\alpha_{ij}$$

$$= s_{ij} m_{ij} d\lambda - \alpha_{ij} m_{ij} d\lambda + \frac{3c_r}{2h_a} \sqrt{\frac{2}{3} m_{rs} m_{rs}} \alpha_{ij} \alpha_{ij} d\lambda$$
(11)

¹⁴⁷ Note that the last term on the right hand side of Equation (11) is non-negative. Substituting the ¹⁴⁸ expression of plastic flow m_{ij} , Equation (8), into Equation (11):

$$\phi \ge (s_{ij} - \alpha_{ij})m_{ij}d\lambda$$

$$= \frac{(s_{ij} - \alpha_{ij})(s_{ij} - \alpha_{ij})}{\sqrt{(s_{mn} - \alpha_{mn})(s_{mn} - \alpha_{mn})}}d\lambda$$

$$= \sqrt{(s_{ij} - \alpha_{ij})(s_{ij} - \alpha_{ij})}d\lambda$$

$$= \sqrt{\frac{2}{3}}kd\lambda \ge 0$$
(12)

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According to Equation (12), the plastic energy dissipation density in the case of (associated) von
 Mises plasticity is always non-negative, which means that the energy dissipation computation does
 follow the second law of thermodynamics.

153 Energy Computation for Associated and Non-Associated Drucker-Prager Plasticity

¹⁵⁴ Drucker-Prager type plasticity is commonly used to model pressure-dependent material behav-¹⁵⁵ ior. In this section, the plastic free energy and plastic dissipation are derived for both associated ¹⁵⁶ and non-associated Drucker-Prager plasticity models. It will be shown that the plastic free energy ¹⁵⁷ function needs an additional pressure-related term, so that the plastic dissipation calculated for ¹⁵⁸ pressure dependent materials is thermodynamically correct.

159 Associated Drucker-Prager Plasticity

¹⁶⁰ The Drucker-Prager yield function is

$$f = \sqrt{(s_{ij} - p\alpha_{ij})(s_{ij} - p\alpha_{ij})} - \sqrt{\frac{2}{3}}kp$$
(13)

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where $p = -(1/3)\sigma_{kk}$ is the mean stress, or hydrostatic pressure, applied on the material. The negative sign ensures that the pressure p is positive when the material is under compression. Note that in Drucker-Prager plasticity, the internal variables k and α_{ij} are dimensionless, while they have the dimension of stress in von Mises plasticity.

¹⁶⁶ The associated plastic flow is:

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$$m_{ij} = \frac{\partial f}{\partial \sigma_{ij}} = \frac{(s_{ij} - p\alpha_{ij}) + \frac{1}{3}\delta_{ij}\alpha_{pq}(s_{pq} - p\alpha_{pq})}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} + \sqrt{\frac{2}{27}}k\delta_{ij}$$
(14)

where δ_{ij} is the Kronecker delta. Due to the pressure term in yield function, the plastic flow has both deviatoric and volumetric components:

$$m_{ij}^{dev} = \frac{s_{ij} - p\alpha_{ij}}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}}$$

$$m_{ij}^{vol} = \frac{\delta_{ij}\alpha_{pq}(s_{pq} - p\alpha_{pq})}{3\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} + \sqrt{\frac{2}{27}}k\delta_{ij}$$
(15)

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Equations (13) and (14) show that the kinematic hardening for associated/non-associated Drucker-Prager plasticity is of the rotational type, which means that the cone representing the yield function in stress space rotates around the origin, as the back stress $p\alpha_{ij}$ evolves. Figure 1 illustrates the Drucker-Prager yield surface with associated plastic flow and rotational kinematic hardening in stress space. Note that the stress tensor σ_{ij} and the plastic flow m_{ij} , or incremental plastic strain tensor ϵ_{ij}^{pl} , are always orthogonal.

Using Equations (7) and (15), the incremental plastic work for associated Drucker-Prager

plasticity becomes 178

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$$\sigma_{ij}d\epsilon_{ij}^{pl} = (s_{ij} - p\delta_{ij})(m_{ij}^{dev} + m_{ij}^{vol})d\lambda$$

$$= (s_{ij}m_{ij}^{dev} - pm_{ii}^{vol})d\lambda$$

$$= \left[\frac{s_{ij}(s_{ij} - p\alpha_{ij})}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} - \frac{p\alpha_{pq}(s_{pq} - p\alpha_{pq})}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} - \sqrt{\frac{2}{3}}kp\right]d\lambda \qquad (16)$$

$$= \left[\sqrt{(s_{ij} - p\alpha_{ij})(s_{ij} - p\alpha_{ij})} - \sqrt{\frac{2}{3}}kp\right]d\lambda = 0$$

Equation 16 proves that the incremental plastic work for associated Drucker-Prager plasticity is 180 always zero. This conclusion is consistent with the orthogonality between the stress tensor σ_{ii} and 181 the incremental plastic strain tensor ϵ_{ii}^{pl} , as shown in Figure 1. However, plastic work is expected to 182 evolve as plastic strain develops in an inelastic material. Thus, associated Drucker-Prager plasticity 183 is thermodynamically inappropriate to be used to model pressure-dependent inelastic materials. 184

Non-Associated Drucker-Prager Plasticity 185

As stated by (Collins and Houlsby 1997), non-associated plastic flow rule comes naturally for 186 a pressure-dependent frictional material. In this section, non-associated Drucker-Prager plasticity, 187 expressed in the classic elastoplasticity form, is discussed from the perspective of energy dissipation. 188 One form of the non-associated plastic flow for pressure-dependent sand material was given by 189 (Manzari and Dafalias 1997): 190

$$m_{ij} = \frac{s_{ij} - p\alpha_{ij}}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} - \frac{1}{3}D\delta_{ij}$$
(17)

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$$m_{ij} = \frac{s_{ij} - p\alpha_{ij}}{\sqrt{(s_{rs} - p\alpha_{rs})(s_{rs} - p\alpha_{rs})}} - \frac{1}{3}D\delta_{ij}$$
(17)

with 192

$$D = \xi \left(\sqrt{\frac{2}{3}} k_d - \frac{\sqrt{s_{mn} s_{mn}}}{p} \right) \tag{18}$$

where ξ and k_d are material model constants that controls the volumetric part of the plastic flow. 194 Note that the plastic flow becomes purely deviatoric when the constant $\xi = 0$. 195

During loading, both the deviatoric and volumetric components of the plastic flow will con-196

tribute to the evolution of plastic work. It has been discussed by (Palmer 1967; Jefferies 1997;
Collins and Muhunthan 2003) that the plastic strain due to isotropic compression leads to the
change of fabric in granular materials. This means that the volumetric part of the plastic strain
should be related to the rise of plastic free energy. Thus, the incremental plastic free energy for
Drucker-Prager plasticity can be calculated from

$$d\psi^{pl} = \left(\frac{3}{2h_a}\alpha_{ij}d\alpha_{ij} - m_{ii}^{vol}d\lambda\right)p\tag{19}$$

Note that the main differences between Equation 19, for Drucker-Prager plasticity, and Equation 10,
 for von Mises plasticity, are the pressure dependency and an additional term for volumetric plastic
 flow.

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Armstrong-Frederick nonlinear kinematic hardening is considered again:

$$d\alpha_{ij} = \left(\frac{2}{3}h_a m_{ij}^{dev} - c_r \alpha_{ij} \sqrt{\frac{2}{3}m_{rs}^{dev} m_{rs}^{dev}}\right) d\lambda \tag{20}$$

²⁰⁸ Note that the evolution of the internal variable α_{ij} is only related to the deviatoric part of the plastic ²⁰⁹ flow. As a result, the internal variable α_{ij} is a deviatoric tensor.

Combining Equations (5), (19), and (20), the incremental plastic dissipation density for nonassociated Drucker-Prager plasticity with Armstrong-Frederick kinematic hardening can be calculated as the following:

$$\phi = (s_{ij}m_{ij}^{dev} - pm_{ii}^{vol})d\lambda - \left(\frac{3}{2h_a}\alpha_{ij}d\alpha_{ij} - m_{ii}^{vol}d\lambda\right)p$$

$$= (s_{ij} - p\alpha_{ij})m_{ij}^{dev}d\lambda + \frac{3c_r}{2h_a}\sqrt{\frac{2}{3}}m_{rs}^{dev}m_{rs}^{dev}\alpha_{ij}\alpha_{ij}pd\lambda$$

$$\geq \sqrt{(s_{ij} - p\alpha_{ij})(s_{ij} - p\alpha_{ij})}d\lambda$$

$$= \sqrt{\frac{2}{3}}kpd\lambda \geq 0$$
(21)

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According to Equation (21), the plastic energy dissipation density in the case of non-associated

Drucker-Prager plasticity is always non-negative. This is important because Equation (21) shows
 that the presented energy calculation methodology for pressure-dependent inelastic material follows
 the second law of thermodynamics.

218 NUMERICAL EXAMPLES

²¹⁹ Constitutive level numerical examples are used to illustrate the presented energy analysis ²²⁰ methodology. Simulations presented in this paper were conducted using the Real-ESSI Simulator ²²¹ (Jeremić et al. 2019), a software, hardware and documentation system for high performance, time ²²² domain, linear or nonlinear/inelastic, deterministic or probabilistic, finite element modeling and ²²³ simulation of soil, structure, and their interaction (http://real-essi.info/). Constitutive ²²⁴ level integrations were performed using the backward Euler algorithm, ensuring the convergence ²²⁵ of stress and yield function. Strain-controlled loading was used in all examples.

Pressure-dependent materials modeled using non-associated Drucker-Prager plasticity under different loading conditions are investigated. The size of the yield cone is chosen to be small so that the material begins to yield, and dissipate energy, at a low shear strain level. Similar assumptions have been made in a number of constitutive models for pressure-dependent materials (Manzari and Dafalias 1997; Taiebat and Dafalias 2008; Pisanò and Jeremić 2014). Armstrong-Frederick kinematic hardening, defined by Equation 20, is used to model strain-hardening/softening behavior.

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Undrained and Drained Triaxial Tests

The first set of numerical tests are conducted for the purpose of model parameter calibration. Two widely-accepted triaxial experiments on Toyoura sand by Verdugo and Ishihara (1996) and Sacramento river sand by Lee and Seed (1967), respectively, are used. The experimental data is obtained from a paper by Taiebat and Dafalias (2008). In this section, the strain-controlled triaxial loading scheme proposed by Bardet and Choucair (1991) is used for all simulation cases.

Toyoura sand is a cohesionless soil consisting of sub-round to sub-angular quartzite particles. Verdugo and Ishihara (1996) reported a series of undrained and drained triaxial tests on isotropically consolidated Toyoura sand samples. In this study, the undrained test results of samples under 100 kPa, 1000 kPa, and 2000 kPa confining pressure are used for model parameter calibration.

Figure 2 shows the comparison between experimental and numerical undrained triaxial test results 242 on Toyoura sand. It can be seen that the calibrated numerical model can represent the material 243 behavior quite well in p-q space, while not that good in q- ϵ_{axial} space, due to simplicity of the 244 used models. The transition from compressive behavior to dilative behavior, which is a key feature 245 of granular materials, is properly modeled. It should be mentioned that the material model used 246 here, non-associated Drucker-Prager plasticity with Armstrong-Frederick kinematic hardening, is 247 simplistic and used to illustrate energy dissipation calculations and not to perfectly match material 248 response. 249

The experimental results of Sacramento river sand conducted by Lee and Seed (1967) have 250 been used to calibrate and validate constitutive models in a number of studies (Bardet and Choucair 251 1991; Taiebat and Dafalias 2008; Ching et al. 2016). In this paper, the drained triaxial test results 252 of Sacramento river sand under 290 kPa, 590 kPa, and 1030 kPa confining pressure, respectively, 253 are used to calibrate numerical model parameters. Figure 3 shows the experimental and numerical 254 drained triaxial test results on Sacramento river sand. It is observed that the volumetric strain 255 behavior and deviatoric stress response of the numerical tests correspond well with those from 256 the physical experiments. For the calibrated parameters, the numerical model shows particularly 257 good performance for the samples under low confining pressure. When the confining pressure is 258 relatively high, the numerical results are still acceptable, especially for small strains. 259

Table 1 shows the calibrated parameters for the material models, which are implemented in Real-ESSI, used in this study. Notice that the main differences between the two material models are the non-associated plastic flow parameters ξ and k_d , as well as the hardening parameters h_a and c_r . This is because a small elastic region was chosen so that the post-yield behavior of the numerical model is dominated by plastic flow and hardening. These two sets of parameters are used in cases presented in the following sections with various loading conditions, in order to investigate the plastic energy dissipation for pressure-dependent materials.

267 Uniaxial Monotonic and Cyclic Shear Loading

For Drucker-Prager plasticity, deviatoric loading will lead to yielding and plastic energy dis-268 sipation. Energy dissipation caused by uniaxial monotonic shear and cyclic shear loading are 269 investigated in this section. Since the material model is pressure-dependent, the relationship be-270 tween initial confining pressure p_0 , shear stress evolution, and plastic energy dissipation is of 271 particular interest. Note that the shear strain in the loading direction follows a linear monotonic or 272 cyclic path, while all other strain components are fixed to simulate undrained shearing condition, 273 as shown in Figure 4 (a). This means that the volumetric strain of the material remains constant, 274 and dilative material behavior will lead to increase in confining pressure. 275

Figure 5 shows the stress-strain responses and energy dissipation results for Toyoura sand and Sacramento river sand material models when uniaxial monotonic shearing is applied. The two materials share very similar shear stress evolution and energy dissipation patterns. Both materials are dilative at large shear strains, which means that the confining pressure keeps increasing as the shearing progresses. This leads to the observed continuous increase of shear stress even after the kinematic hardening internal variable α_{ij} reaches saturation. Also, as expected, the sample under a higher level of confinement develops a larger shear stress.

The plastic dissipation density plots in Figure 5 present an interesting relationship between 283 plastic dissipation and confining pressure. When the initial confining pressure is increased from 284 200 kPa to 1000 kPa, more energy dissipation is observed. However, when the confining pressure 285 is raised to 2000 kPa, the plastic dissipation density at low strain level is observed to be smaller 286 than those in the other two cases. This is because when a pressure-dependent material is under a 287 larger confinement, it can resist a higher level of shear stress before significant yielding. Then, after 288 the shear strain becomes large enough, the more-confined material yields and dissipates energy 289 at a higher rate due to its larger shear stress. Such energy dissipation feature could be important 290 when modeling pressure-dependent materials, including soils, mine tailings, and other granular 291 materials. 292

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Figure 6 shows the stress-strain responses and energy dissipation results of the Toyoura sand

and Sacramento river sand under uniaxial cyclic shear loading. The responses of the two materials
 again have very similar patterns under cyclic shear loading. The magnitude of shear strain is
 increased for each loading cycle, and as a result, the shear stress keeps growing.

The plastic dissipation density rate is always positive, which means that the presented energy 297 analysis methodology follows the second law of thermodynamics, as proven in Equation 21. Notice 298 that the strain energy density is not zero at the beginning due to initial confinement. For the first few 299 cycles with small shear strain magnitudes, the accumulated plastic dissipation density is smaller 300 than strain energy density. Then as the applied shear strain increases, plastic dissipation density 301 rapidly increases and surpasses strain energy density. This means that the majority of input work 302 will be dissipated due to material inelasticity when an inelastic material is loaded with a number 303 of deviatoric loading cycles. 304

Biaxial Shear Loading

The next example focuses on the material response and plastic energy dissipation when biaxial shear loading is applied. The strain-controlled loading setup of biaxial shear is shown in Figure 4 (b). Compared to uniaxial loading, biaxial shearing condition is one step closer to the realistic, fully three-dimensional loading condition. In addition, biaxial shearing test is a common type of laboratory experiments on granular materials.

The previous examples have shown that the two materials, Toyoura sand and Sacramento river sand, share similar mechanical and energy responses when loaded in shear. Therefore, only Toyoura sand is investigated in this section. Figure 7 shows the stress-strain responses and energy computation results for the material under biaxial shear loading. As shown in the shear strain path in Figure 7, the full loading cycle consists of four loading branches:

- ³¹⁶ (1) Increase shear strain ϵ_{xy} from 0 to 5%;
- ³¹⁷ (2) Increase shear strain ϵ_{xz} from 0 to 5%;
- 318 (3) Decrease shear strain ϵ_{xy} from 5% to 0;
- ³¹⁹ (4) Decrease shear strain ϵ_{xz} from 5% to 0.

The initial confining stress is 1000 kPa. Due to material dilatancy and the constant volume 320 loading condition, the confining pressure evolves as the shearing progresses. This is the reason 321 why the stress state of the material at the end of a full loading cycle is different than its original 322 hydrostatic state before loading, as can be observed in the stress path in the principal stress space. 323 From the shear stress-shear strain plots in the xy and xz directions shown in Figure 7, it is seen 324 that shear loading in one direction does influence the stress response in the other direction. For 325 example, during loading branch (2), the increase of shear strain in the xz direction not only leads to 326 an increase of shear stress in the same direction, but also caused the shear stress in the xy direction 327 to drop. 328

For pressure-dependent frictional material under deviatoric loading, evolution of fabric and dissipative slipping between particles are always occurring, even during unloading. According to the plastic dissipation density plot, positive dissipation rates are observed in all four loading branches.

333 CONCLUSIONS

This paper presented a thermodynamics-based energy analysis approach for pressure-dependent materials. Theoretical formulation of plastic energy dissipation in pressure-dependent, nonassociated Drucker-Prager plasticity model was derived and discussed. The proposed energy computation method was implemented in the Real-ESSI simulator system, and illustrated on a series of numerical examples. It was also shown that the presented energy analysis approach can be used in large-scale finite element simulation of 3D dynamic inelastic system.

The energy analysis equations were derived based on thermomechanics with proper assumptions. The difference between plastic work and plastic dissipation, as well as the importance of plastic free energy, was highlighted. Pressure-independent von Mises plasticity was examined from the perspective of energy storage and dissipation. It was mathematically proven that the energy formulation from Yang et al. (2018a) does follow the first and second law of thermodynamics.

Next, Drucker-Prager plasticity model with rotational kinematic hardening was discussed with focus on energy dissipation behavior. A close examination of associated Drucker-Prager plasticity

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showed that zero plastic work is always obtained due to the orthogonality between stress tensor and
 incremental plastic strain tensor. Thus, it was concluded that associated Drucker-Prager plasticity
 is a thermodynamically inappropriate constitutive model.

The plastic free energy function for von Mises plasticity was modified to incorporate the influence of confinement on the energy behavior of non-associated Drucker-Prager material model. Based on experimental and theoretical works by a number of researchers, it was assumed that the volumetric part of the plastic strain is related to the rise of plastic free energy. The energy formulation derived based on this assumption was then proven to always give non-negative incremental plastic dissipation, as required by the second law of thermodynamics.

Presented energy computation approach was illustrated using numerical examples of pressuredependent material models under different loading conditions. Two sets of model parameters were calibrated using the triaxial test data on Toyoura sand (Verdugo and Ishihara 1996) and Sacramento river sand (Lee and Seed 1967). Uniaxial shearing examples showed that the proposed analysis method can properly account for the influence of pressure-dependency on the energy dissipation behavior of Drucker-Prager model. In general, it was observed that the majority of input work will be dissipated if significant cyclic deviatoric loading is applied.

In the case of biaxial shear loading, it was observed that the evolution of shear stress in one direction influenced the shear stress response in the other direction, due to the pressure dependency of the material model. This also lead to different energy dissipation behaviors when loads were increased or decreased in different directions. More importantly, it was pointed out that the continuous dissipation of mechanical energy in pressure-dependent frictional materials was properly modeled by the proposed energy analysis methodology.

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371 **REFERENCES**

Armstrong, P. and Frederick, C. (1966). "A mathematical representation of the multiaxial bauschinger effect.." *Technical Report RD/B/N/ 731*,, C.E.G.B.

- Bardet, J. P. and Choucair, W. (1991). "A linearized integration technique for incremental constitu tive equations." *International Journal for Numerical and Analytical Methods in Geomechanics*, 15(1), 1–19.
- ³⁷⁷ Ching, J., Lin, G.-H., Chen, J.-R., and Phoon, K.-K. (2016). "Transformation models for effective ³⁷⁸ friction angle and relative density calibrated based on generic database of coarse-grained soils."
- 379 *Canadian Geotechnical Journal*, 54(4), 481–501.
- Collins, I. (2002). "Associated and non-associated aspects of the constitutive laws for coupled elastic/plastic materials." *International Journal of Geomechanics*, 2(2), 259–267.
- Collins, I. (2003). "A systematic procedure for constructing critical state models in three dimensions." *International Journal of Solids and Structures*, 40(17), 4379–4397.
- Collins, I. and Kelly, P. (2002). "A thermomechanical analysis of a family of soil models." *Geotechnique*, 52(7), 507–518.
- Collins, I. and Muhunthan, B. (2003). "On the relationship between stress–dilatancy, anisotropy, and plastic dissipation for granular materials." *Geotechnique*, 53(7), 611–618.
- Collins, I. F. and Hilder, T. (2003). "A theoretical framework for constructing elastic/plastic constitutive models from triaxial tests." *International Journal for Numerical an Analytical Methods in Geomechanics*, 26, 1313–1347.
- ³⁹¹ Collins, I. F. and Houlsby, G. T. (1997). "Application of thermomechanical principles to the ³⁹² modelling of geotechnical materials." *Proceedings of Royal Society London*, 453, 1975–2001.
- ³⁹³ Dafalias, Y. and Popov, E. (1975). "A model of nonlinearly hardening materials for complex ³⁹⁴ loading." *Acta mechanica*, 21(3), 173–192.
- Farren, W. and Taylor, G. (1925). "The heat developed during plastic extension of metals." *Proceedings of the royal society of London A: mathematical, physical and engineering sciences*, The
 Royal Society, 107(743), 422–451.
- ³⁹⁸ Feigenbaum, H. P. and Dafalias, Y. F. (2007). "Directional distortional hardening in metal plasticity
- within thermodynamics." *International Journal of Solids and Structures*, 44(22-23), 7526–7542.
- Jefferies, M. (1997). "Plastic work and isotropic softening in unloading." *Géotechnique*, 47(5).

- Jeremić, B., Jie, G., Cheng, Z., Tafazzoli, N., Tasiopoulou, P., Pisanò, F., Abell, J. A., Watanabe,
 K., Feng, Y., Sinha, S. K., Behbehani, F., Yang, H., and Wang, H. (1989-2019). *The Real ESSI / MS ESSI Simulator System*. University of California, Davis and Lawrence Berkeley National
 Laboratory. http://real-essi.info/.
- Lee, K. L. and Seed, H. B. (1967). "Drained strength characteristics of sands." *Journal of Soil Mechanics & Foundations Div.*
- ⁴⁰⁷ Lubliner, J. (1990). *Plasticity Theory*. Macmillan Publishing Company, New York.
- Luong, M. P. (1986). "Characteristic threshold and infrared vibrothermography of sand." *Geotechnical Testing Journal*, 9(2), 80–86.
- Manzari, M. T. and Dafalias, Y. F. (1997). "A critical state two–surface plasticity model for sands."
 Géotechnique, 47(2), 255–272.
- ⁴¹² Okada, N. and Nemat-Nasser, S. (1994). "Energy dissipation in inelastic flow of saturated cohe-⁴¹³ sionless granular media." *Geotechnique*, 44(1), 1–19.
- Palmer, A. C. (1967). "Stress-strain relations for clays: an energy theory." *Géotechnique*, 17(4),
 348–358.
- Pisanò, F. and Jeremić, B. (2014). "Simulating stiffness degradation and damping in soils via a
 simple visco-elastic-plastic model." *Soil Dynamics and Geotechnical Earthquake Engineering*,
 63, 98–109.
- Rittel, D. (2000). "An investigation of the heat generated during cyclic loading of two glassy
 polymers. part i: Experimental." *Mechanics of Materials*, 32(3), 131–147.
- Rosakis, P., Rosakis, A., Ravichandran, G., and Hodowany, J. (2000). "A thermodynamic internal
 variable model for the partition of plastic work into heat and stored energy in metals." *Journal of the Mechanics and Physics of Solids*, 48(3), 581–607.
- Schofield, A. and Wroth, P. (1968). *Critical state soil mechanics*, Vol. 310. McGraw-Hill London.
- Taiebat, M. and Dafalias, Y. F. (2008). "SANISAND: Simple anisotropic sand plasticity model."
 International Journal for Numerical and Analytical Methods in Geomechanics (in print, available
 in earlyview).

428	Taylor, G. I. and Quinney, H. (1934). "The latent energy remaining in a metal after cold working."
429	Proceedings of the Royal Society of London. Series A, Containing Papers of a Mathematical and
430	Physical Character, 143(849), 307–326.

- Verdugo, R. and Ishihara, K. (1996). "The steady state of sandy soils." *Soils and foundations*, 36(2),
 81–91.
- Veveakis, E., Vardoulakis, I., and Di Toro, G. (2007). "Thermoporomechanics of creeping land slides: The 1963 valont slide, northern Italy." *Journal of Geophysical Research: Earth Surface*, 112(F3).
- Yang, H., Sinha, S. K., Feng, Y., McCallen, D. B., and Jeremić, B. (2018a). "Energy dissipation
 analysis of elastic-plastic materials." *Computer Methods in Applied Mechanics and Engineering*,
 331, 309–326.
- Yang, H., Wang, H., Feng, Y., Wang, F., and Jeremić, B. (2018b). "Energy dissipation in solids
 due to material inelasticity, viscous coupling, and algorithmic damping." *ASCE Journal of Engineering Mechanics* In Print.
- ⁴⁴² Ziegler, H. and Wehrli, C. (1987). "The derivation of constitutive relations from the free energy
- and the dissipation function." *Advances in applied mechanics*, 25, 183–238.

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Parameter	Unit -	Material	
Tarameter		Toyoura	Sacramento
mass_density (ρ)	kg/m^3	2000	2000
<pre>elastic_modulus(E)</pre>	MPa	25.0	150.0
poisson_ratio(v)		0.3	0.3
druckerprager_k		0.107	0.107
$armstrong_frederick_ha(h_a)$	MPa	17.5	45.0
armstrong_frederick_cr(c _r)		150	300
plastic_flow_xi(ξ)		1.9	0.7
$plastic_flow_kd(k_d)$		0.92	0.90

TABLE 1. Material model parameters of the pressure-dependent materials used in this study.

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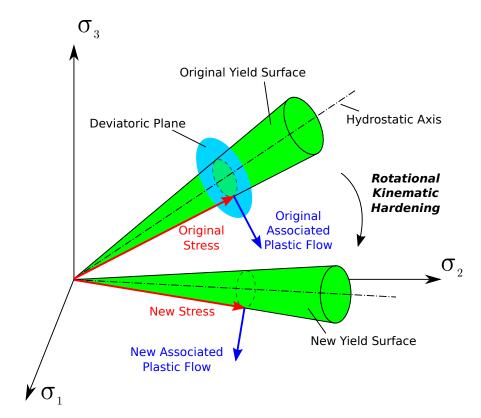


Fig. 1. Drucker-Prager yield surface with associated plastic flow and rotational kinematic hardening in stress space.

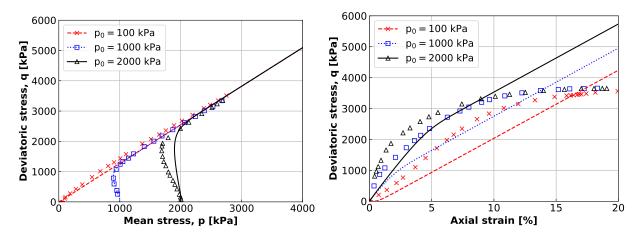


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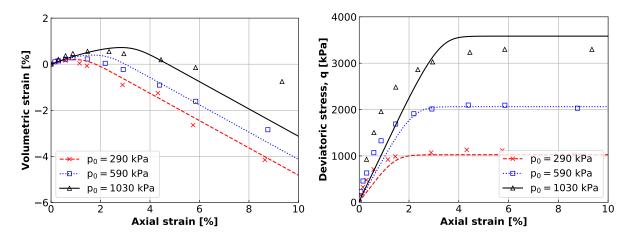


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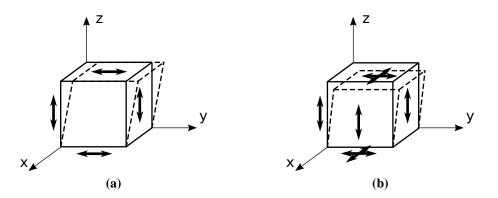


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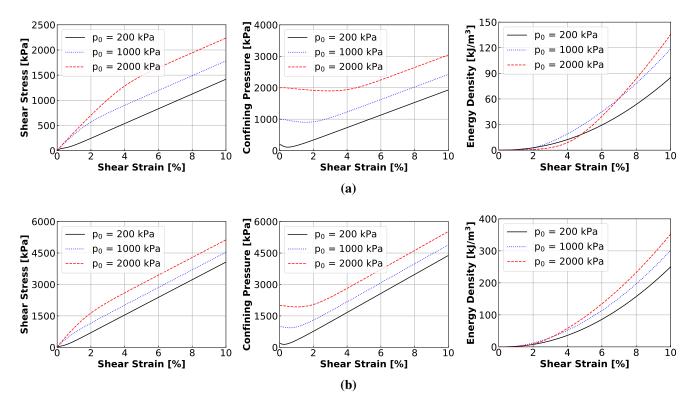


Fig. 5. Stress-strain responses and plastic energy dissipation results of pressure-dependent materials under uniaxial monotonic shear loading: (a) Toyoura sand; (b) Sacramento river sand.

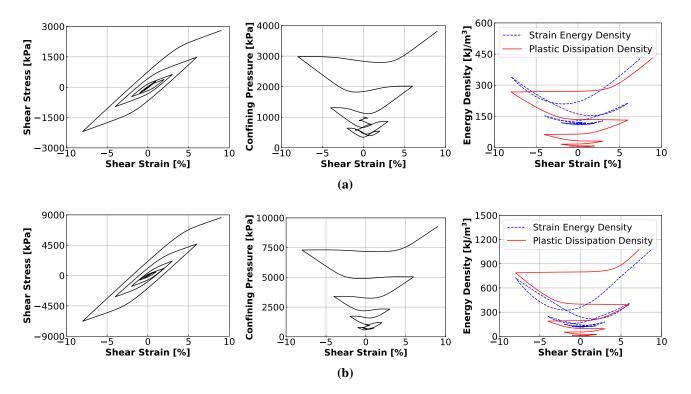


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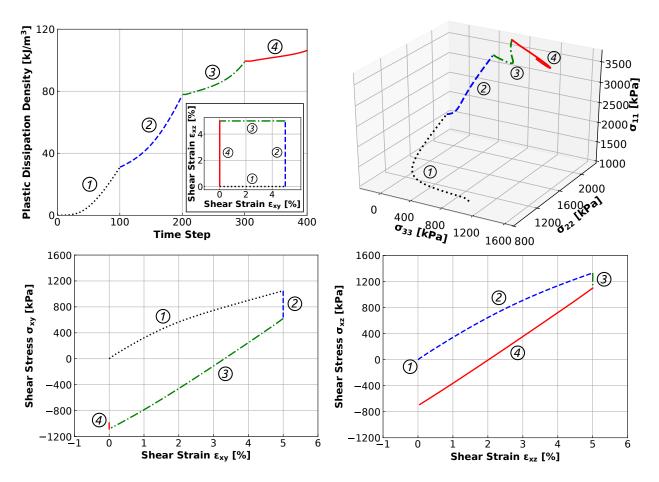


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