

The Role of Material Variability and Uncertainty in Elastic-Plastic Finite Element Simulations

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Outline

Motivation

- Historical Overview
- Uncertainties in Material

Boundary Value Problem

- Propagation of Uncertainties in Mechanics (Geomechanics)
- Stochastic Finite Element Method

Probabilistic Elasto–Plasticity

- Probabilistic Elastic–Plastic: Differential Equation
- Probabilistic Elastic–Plastic Response

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Brownian Motions

- ▶ Governing (Langevin) equation:

$$m \frac{dv}{dt} = F(x) - \beta v + \eta(t)$$

- ▶ Probability density function (PDF) of particle displacement obeys a simple diffusion equation (Einstein (1905)):

$$\frac{\partial f(x, t)}{\partial t} = D \frac{\partial^2 f(x, t)}{\partial x^2}$$

- ▶ Addition of external forces (gravity, elastic or magnetic attraction) → Fokker-Planck-Kolmogorov (FPK) equation governs the PDF (Kolmogorov 1941)
- ▶ Alternately, Monte Carlo method can be used for solution of Langevin equation → computationally very expensive

Stochastic Systems: Random Forcing

- ▶ Classical approach: relationship between the autocorrelation function and spectral density function (Wiener 1930) → Paved the way to the solution of general stochastic differential equation (SDE)
- ▶ SDEs with random forcing → Highly developed mathematical theory for Itô type equation:

$$dx = a(x, t)dt + b(x, t)dW$$

- ▶ Solution is a Markov process
- ▶ PDF of solution process satisfies a FPK PDE

$$\frac{\partial p(x, t)}{\partial t} = -\frac{\partial}{\partial x} [a(x, t)p(x, t)] + \frac{1}{2} \frac{\partial^2}{\partial x^2} [b^2(x, t)p(x, t)]$$

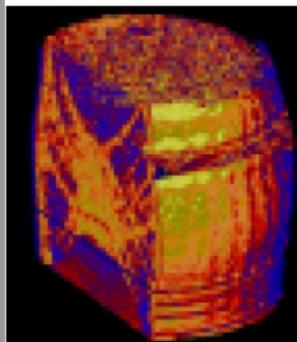
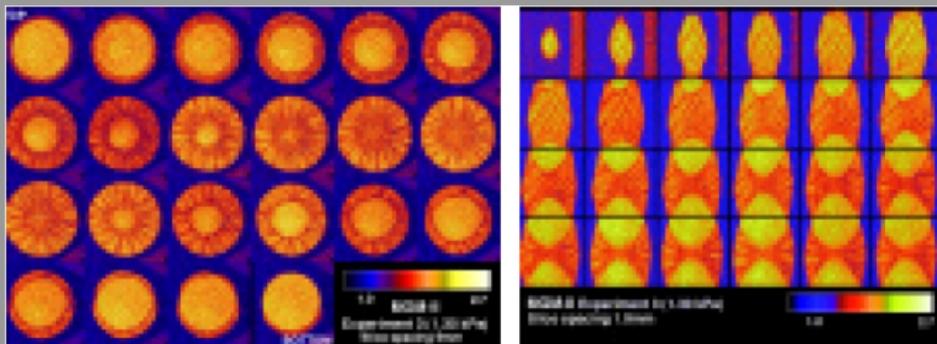
Stochastic Systems: Random Coefficient

- ▶ Approximate Solution methods
 - ▶ Functional integration approach (Hopf 1952)
 - ▶ Averaged equation approach (Bharrucha-Reid 1968)
 - ▶ Numerical approaches
 - ▶ Monte Carlo method
- ▶ FPK equation for the characteristic functional of the solution for problem of wave propagation in random media (Lee 1974)
- ▶ Eulerian-Lagrangian form of FPK equation for probabilistic solution of flow through porous media (Kavvas 2003)

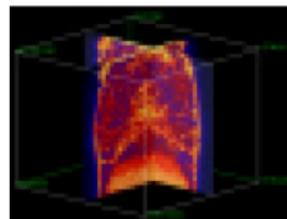
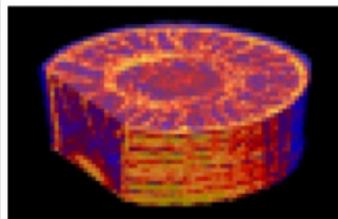
Material Uncertainties

- ▶ Material's (concrete, metals, soil, rock, bone, foam, powder etc.) behavior is inherently uncertain
 - ▶ Spatial variability
 - ▶ Point-wise uncertainty - testing error, transformation error
- ▶ Failure mechanisms related to spatial variability (strain localization and bifurcation of response)
- ▶ Inverse problems
 - ▶ New material design, (*point-wise*)
 - ▶ Solid and/or structure design (or retrofits), (*spatial*)

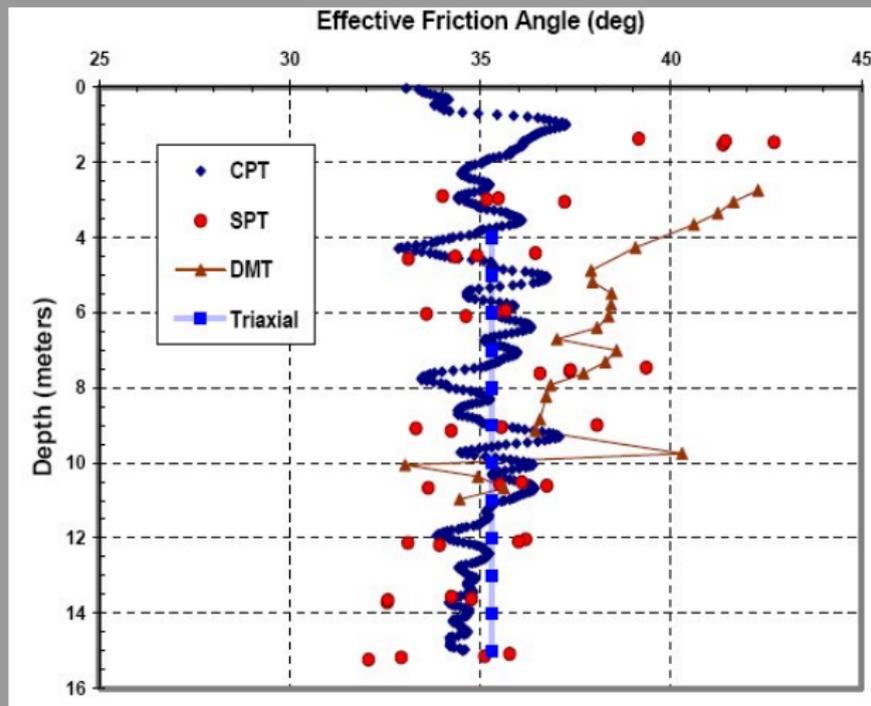
Soil: Inside Failure (MGM)



Computed tomography (CT) images of resin-impregnated MGM specimens (above), are assembled to provide 3-D volume renderings (below) of density patterns formed by diffused bifurcation under the external loading stress profile applied during the experiments.



Soil: Spatial Variation (Mayne et al. (2000))



Soil Uncertainties and Quantifications

- ▶ Natural variability of soil deposit (Fenton 1999) → function of soil formation process
- ▶ Testing error (Stokoe et al. 2004)
 - ▶ Imperfection of instruments
 - ▶ Error in methods to register quantities
- ▶ Transformation error (Phoon and Kulhawy 1999)
 - ▶ Correlation by empirical data fitting (e.g. CPT data → friction angle etc.)

Probabilistic material (Soil Site) Characterization

- ▶ Ideal: complete probabilistic site characterization
- ▶ Large (physically large but not statistically) amount of data
 - ▶ Site specific mean and coefficient of variation (COV)
 - ▶ Covariance structure from similar sites (e.g. Fenton 1999)
- ▶ Minimal data: general guidelines for typical sites and test methods (Phoon and Kulhawy (1999))
 - ▶ COVs and covariance structures of inherent variabilities
 - ▶ COVs of testing errors and transformation uncertainties.
- ▶ Moderate amount of data → Bayesian updating (e.g. Phoon and Kulhawy 1999, Baecher and Christian 2003)

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Propagation of Uncertainties in Mechanics

Governing equation

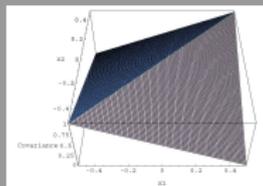
- ▶ Dynamic problems $\rightarrow M\ddot{u} + C\dot{u} + Ku = \phi$
- ▶ Static problems $\rightarrow Ku = \phi$

Existing solution methods

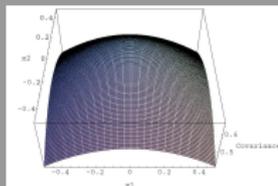
- ▶ **Random r.h.s** (external force random)
 - ▶ FPK equation approach
 - ▶ Use of fragility curves with deterministic FEM
- ▶ **Random l.h.s** (material properties random)
 - ▶ Monte Carlo approach with DFEM \rightarrow CPU expensive
 - ▶ Stochastic finite element method (Perturbation method, fails if COVs of soil $> 20\%$; Spectral method \rightarrow elastic material)

Truncated Karhunen–Loeve Expansion for Input Field

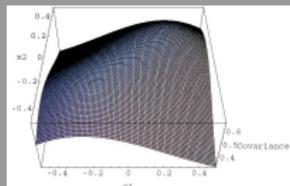
- ▶ Input random fields represented in eigen-modes of covariance kernel
- ▶ Error minimizing property
- ▶ Minimizes number of stochastic dimensions



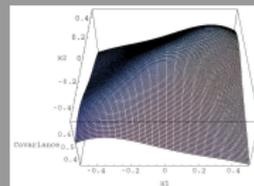
Exact cov. surface



1-term



2-terms



3-terms

Model covariance kernel: $C(x_1, x_2) = e^{-|x_1 - x_2|/b}$

Truncated K-L approximation: $C(x_1, x_2) = \sum_{k=1}^M \lambda_k f_k(x_1) f_k(x_2)$

Polynomial Chaos (PC) Expansion for DOFs

- ▶ DOF covariance kernel is not known a priori (unknown eigenvalues e_j and eigenvectors $b_j(x)$)

$$u(x, \theta) = \sum_{j=1}^L e_j \chi_j(\theta) b_j(x)$$

- ▶ DOFs expressed as functionals of known input random variables and unknown deterministic function

$$u(x, \theta) = \zeta[\xi_i(\theta), x]$$

- ▶ Need a basis of known random variables \rightarrow PC expansion

$$\chi_j(\theta) = \sum_{i=0}^P \gamma_i^{(j)} \psi_i[\{\xi_r\}];$$

$$u(x, \theta) = \sum_{j=1}^L \sum_{i=0}^P \gamma_i^{(j)} \psi_i[\{\xi_r\}] e_j b_j(x) = \sum_{i=0}^P \psi_i[\{\xi_r\}] d_i(x)$$

- ▶ Deterministic coefficients can be found by minimizing norm of error of finite representation (e.g. using Galerkin scheme)

SSEPFEM Formulation

$$\sum_{n=1}^N K_{mn} d_{ni} + \sum_{n=1}^N \sum_{j=0}^P d_{nj} \sum_{k=1}^M C_{ijk} K'_{mnk} = \langle F_m \psi_i[\{\zeta_r\}] \rangle$$

$$K_{mn} = \int_D B_n D B_m dV$$

$$K'_{mnk} = \int_D B_n \sqrt{\lambda_k} h_k B_m dV$$

$$C_{ijk} = \langle \zeta_k(\theta) \psi_i[\{\zeta_r\}] \psi_j[\{\zeta_r\}] \rangle$$

$$F_m = \int_D \phi N_m dV$$

- ▶ Generalized DOF
- ▶ Material (soil) nonlinearity → Constitutive integration at Gauss point → Probabilistic Elasto–Plasticity

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Uncertainty Propagation through Constitutive Eq.

- ▶ General 3-D elastic-plastic constitutive law →

$$\frac{d\sigma_{ij}}{dt} = D_{ijkl} \frac{d\epsilon_{kl}}{dt}$$

$$D_{ijkl} = \begin{cases} D_{ijkl}^{el} & \text{for elastic} \\ D_{ijkl}^{el} - \frac{D_{ijmn}^{el} \frac{\partial U}{\partial \sigma_{mn}} \frac{\partial f}{\partial \sigma_{pq}} D_{pqkl}^{el}}{\frac{\partial f}{\partial \sigma_{rs}} D_{rstu}^{el} \frac{\partial U}{\partial \sigma_{tu}} - \frac{\partial f}{\partial q_*} r_*} & \text{for elastic-plastic} \end{cases}$$

- ▶ Non-linear coupling in the coefficient (elastic-plastic modulus)
- ▶ Focusing on 1-D constitutive Behavior → a nonlinear ODE with random coefficient and random forcing

Previous Works

- ▶ Linear algebraic or differential equations → Analytical solution:
 - ▶ Variable Transformation Method (Montgomery and Runger 2003)
 - ▶ Cumulant Expansion Method (Gardiner 2004)
- ▶ Nonlinear differential equations (elasto-plastic/viscoelastic-viscoplastic):
 - ▶ Monte Carlo Simulation (Schueller 1997, De Lima et al 2001, Mellah et al. 2000, Griffiths et al. 2005...)
 - ▶ Perturbation Method (Anders and Hori 2000, Kleiber and Hien 1992, Matthies et al. 1997)
- ▶ Monte Carlo method: accurate, very costly
- ▶ Perturbation method: first and second order Taylor series expansion about mean - limited to problems having small C.O.V. and inherits 'closure problem'

Problem Statement

- ▶ General 3-D elastic-plastic constitutive law:

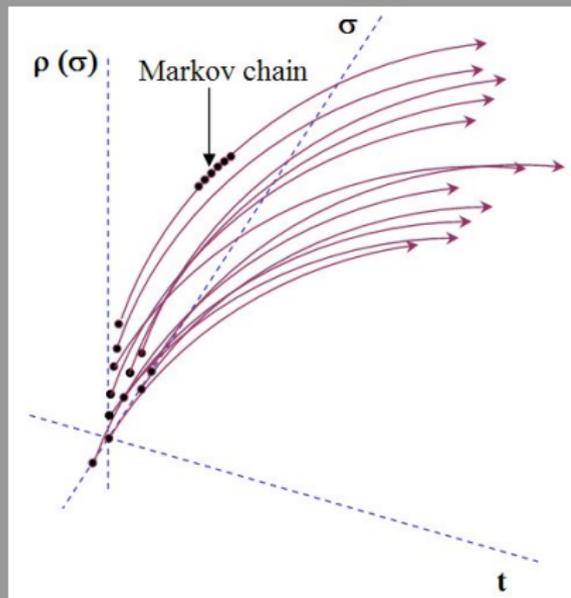
$$d\sigma_{ij} = \left\{ \begin{array}{c} D_{ijkl}^{el} - \frac{D_{ijmn}^{el} \frac{\partial U}{\partial \sigma_{mn}} \frac{\partial f}{\partial \sigma_{pq}} D_{pqkl}^{el}}{\frac{\partial f}{\partial \sigma_{rs}} D_{rstu}^{el} \frac{\partial U}{\partial \sigma_{tu}} - \frac{\partial f}{\partial q_*} r_*} \end{array} \right\} d\epsilon_{kl}$$

- ▶ Focusing on 1-D constitutive Behavior → a nonlinear ODE with random coefficient and random forcing

$$\begin{aligned} \frac{d\sigma(x, t)}{dt} &= \beta(\sigma(x, t), D^{el}(x), q(x), r(x); x, t) \frac{d\epsilon(x, t)}{dt} \\ &= \eta(\sigma, D^{el}, q, r, \epsilon; x, t) \end{aligned}$$

with an initial condition $\sigma(0) = \sigma_0$

Stochastic Continuity (Liouville) Equation



$$\frac{\partial \rho(\sigma(x, t), t)}{\partial t} = -\frac{\partial}{\partial \sigma} \left[\eta(\sigma(x, t), D^{el}(x), q(x), r(x), \epsilon(x, t)) \right] \rho[\sigma(x, t), t]$$

Initial condition:

$$\rho(\sigma, 0) = \delta(\sigma - \sigma_0)$$

$\rho(\sigma, 0) \rightarrow$ density of probabilistic solutions in σ space

Ensemble Average form of Liouville Equation

→ van Kampen's Lemma → $\langle \rho(\sigma, t) \rangle = P(\sigma, t)$, ensemble average of phase density is the probability density;

→ Continuity equation written in ensemble average form (eg. cumulant expansion method (Kavvas and Karakas 1996)):

$$\begin{aligned} \frac{\partial \langle \rho(\sigma(x_t, t), t) \rangle}{\partial t} &= -\frac{\partial}{\partial \sigma} \left[\left\{ \left\langle \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)) \right\rangle \right. \right. \\ &+ \int_0^t d\tau \text{Cov}_0 \left[\frac{\partial \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t))}{\partial \sigma}; \right. \\ &\left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right\rangle \right] \langle \rho(\sigma(x_t, t), t) \rangle \left. \right] \\ &+ \frac{\partial^2}{\partial \sigma^2} \left[\left\{ \int_0^t d\tau \text{Cov}_0 \left[\eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)); \right. \right. \right. \\ &\left. \left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right) \right] \right\} \langle \rho(\sigma(x_t, t), t) \rangle \left. \right] \end{aligned}$$

Eulerian–Lagrangian FPK Equation

$$\begin{aligned}
 \frac{\partial P(\sigma(x_t, t), t)}{\partial t} &= -\frac{\partial}{\partial \sigma} \left[\left\langle \left\langle \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t))) \right\rangle \right\rangle \right. \\
 + \int_0^t d\tau \text{Cov}_0 &\left[\frac{\partial \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)))}{\partial \sigma}; \right. \\
 &\left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right) \right] \left. \right\} P(\sigma(x_t, t), t) \left. \right] \\
 + \frac{\partial^2}{\partial \sigma^2} &\left[\left\langle \int_0^t d\tau \text{Cov}_0 \left[\eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t))); \right. \right. \right. \\
 &\left. \left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right) \right] \right\rangle P(\sigma(x_t, t), t) \left. \right]
 \end{aligned}$$

- ▶ Complete probabilistic description of response
- ▶ Second-order exact to covariance of time
- ▶ Deterministic equation (in probability density space)

Solution of FPK Equation

- ▶ FPK equation → advection-diffusion equation or continuity equation

$$\frac{\partial P(\sigma, t)}{\partial t} = -\frac{\partial}{\partial \sigma} \left[N_{(1)} P(\sigma, t) - \frac{\partial}{\partial \sigma} \{ N_{(2)} P(\sigma, t) \} \right] = -\frac{\partial \zeta}{\partial \sigma}$$

- ▶ Initial condition
 - ▶ Deterministic → Dirac delta function → $P(\sigma, 0) = \delta(\sigma)$
 - ▶ Random → Any given distribution
- ▶ Boundary condition: Reflecting BC → conserves probability mass $\zeta(\sigma, t)|_{At \text{ Boundaries}} = 0$
- ▶ Numerical scheme → *Finite Difference Technique*

Application of FPK equation to Material Models

- ▶ FPK equation is applicable to any incremental elastic–plastic material model (only the coefficients $N_{(1)}$ and $N_{(2)}$ differ)
- ▶ Unique attributes of probabilistic solution
 - ▶ Solution in terms of PDF, not a single value of stress
 - ▶ Influence of initial condition on the PDF of stress
 - ▶ Transition between elastic and elastic–plastic
 - ▶ Symmetry and non–symmetry in PDF of stress
 - ▶ Differences in mean, mode and deterministic solution of stress
 - ▶ Interaction of random soil properties on the PDF of stress

Elastic Response with Random G

- ▶ General form of elastic constitutive rate equation

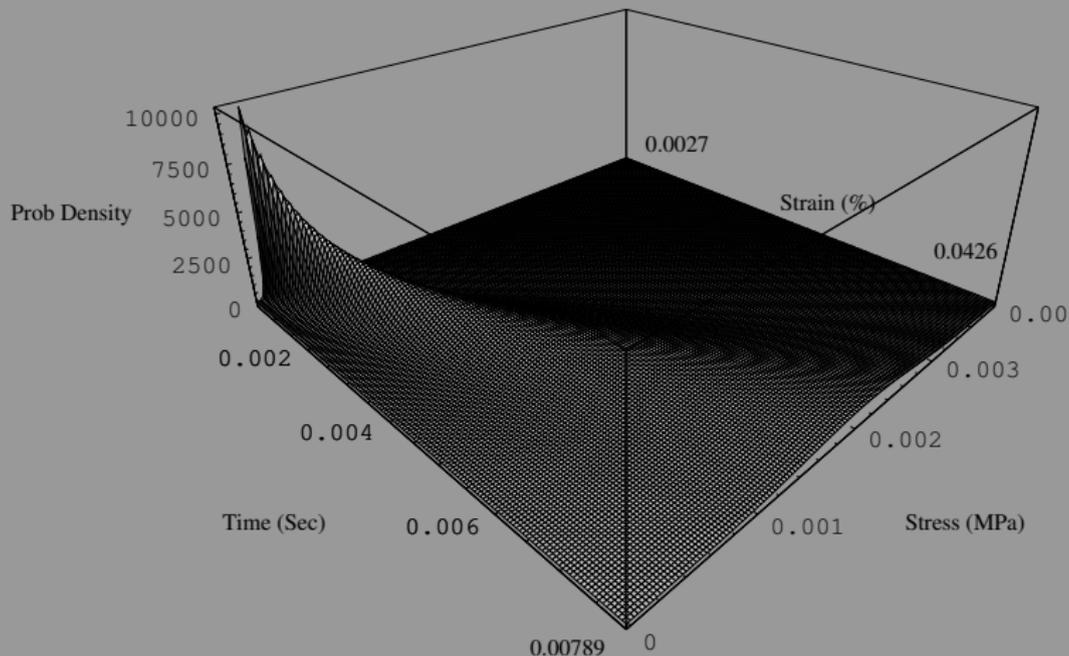
$$\begin{aligned}\frac{d\sigma_{12}}{dt} &= 2G \frac{d\epsilon_{12}}{dt} \\ &= \eta(G, \epsilon_{12}; t)\end{aligned}$$

- ▶ Advection and diffusion coefficients of FPK equation

$$N_{(1)} = 2 \frac{d\epsilon_{12}}{dt} \langle G \rangle$$

$$N_{(2)} = 4t \left(\frac{d\epsilon_{12}}{dt} \right)^2 \text{Var}[G]$$

Elastic Response with Random G



$\langle G \rangle = 2.5 \text{ MPa}$; Std. Deviation $[G] = 0.5 \text{ MPa}$

Drucker-Prager Linear Hardening with Random G

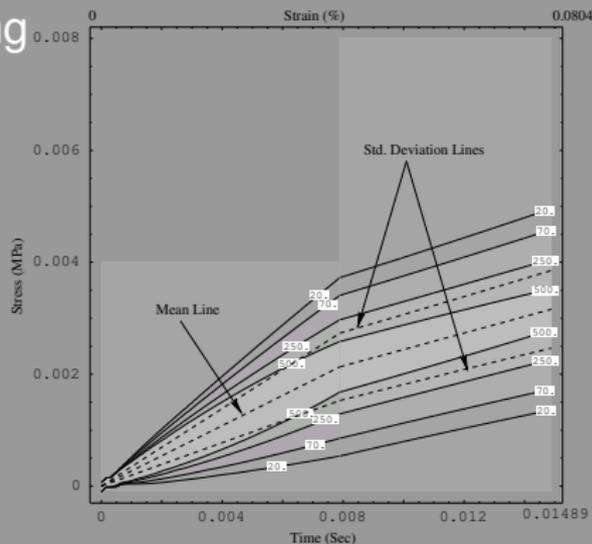
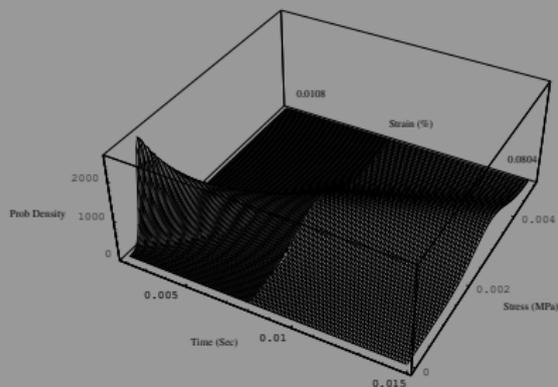
$$\frac{d\sigma_{12}}{dt} = G^{ep} \frac{d\epsilon_{12}}{dt} = \eta(\sigma_{12}, G, K, \alpha, \alpha', \epsilon_{12}; t)$$

Advection and diffusion coefficients of FPK equation

$$N_{(1)} = \frac{d\epsilon_{12}}{dt} \left\langle 2G - \frac{G^2}{G + 9K\alpha^2 + \frac{1}{\sqrt{3}} l_1 \alpha'} \right\rangle$$

$$N_{(2)} = t \left(\frac{d\epsilon_{12}}{dt} \right)^2 \text{Var} \left[2G - \frac{G^2}{G + 9K\alpha^2 + \frac{1}{\sqrt{3}} l_1 \alpha'} \right]$$

Drucker-Prager Linear Hardening with Random G



- ▶ Approximation of I.C.
- ▶ Smooth transition between el. & el.-pl.
- ▶ Symmetry in probability distribution

Modified Cam Clay Constitutive Model

$$\frac{d\sigma_{12}}{dt} = G^{ep} \frac{d\epsilon_{12}}{dt} = \eta(\sigma_{12}, G, M, e_0, p_0, \lambda, \kappa, \epsilon_{12}; t)$$

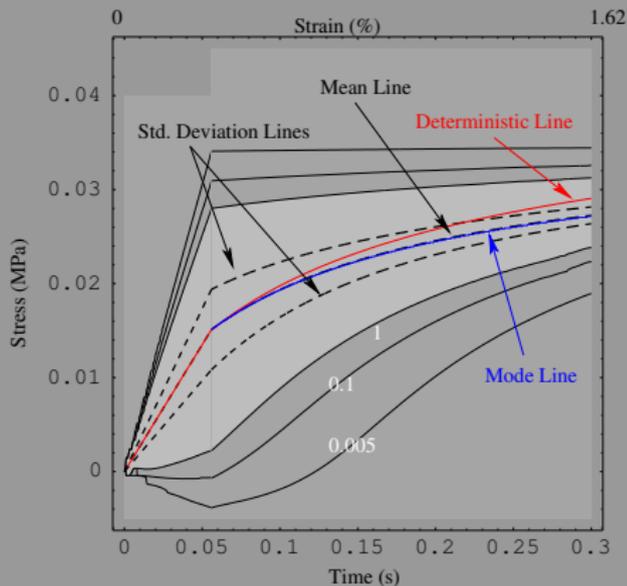
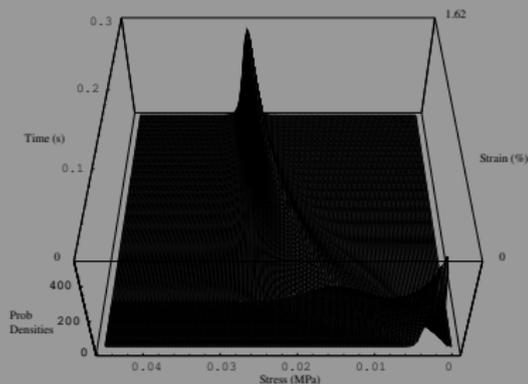
$$\eta = \left[2G - \frac{\left(36 \frac{G^2}{M^4}\right) \sigma_{12}^2}{\frac{(1 + e_0)p(2p - p_0)^2}{\kappa} + \left(18 \frac{G}{M^4}\right) \sigma_{12}^2 + \frac{1 + e_0}{\lambda - \kappa} pp_0(2p - p_0)} \right]$$

Advection and diffusion coefficients of FPK equation

$$N_{(1)}^{(i)} = \langle \eta^{(i)}(t) \rangle + \int_0^t d\tau \text{cov} \left[\frac{\partial \eta^{(i)}(t)}{\partial t}; \eta^{(i)}(t - \tau) \right]$$

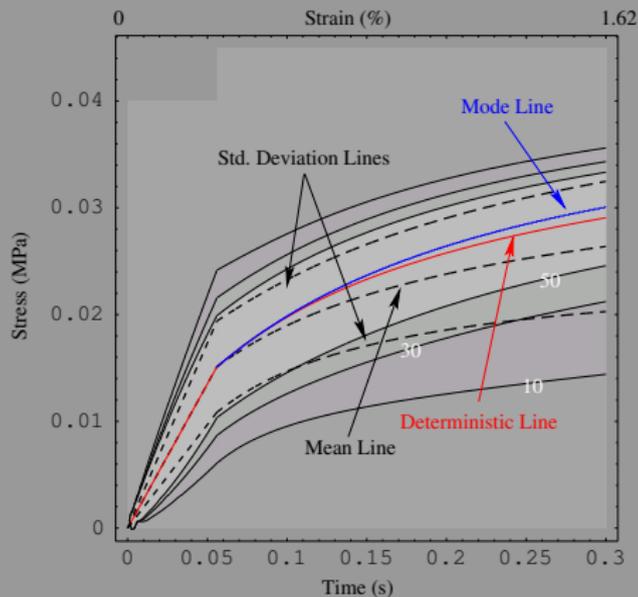
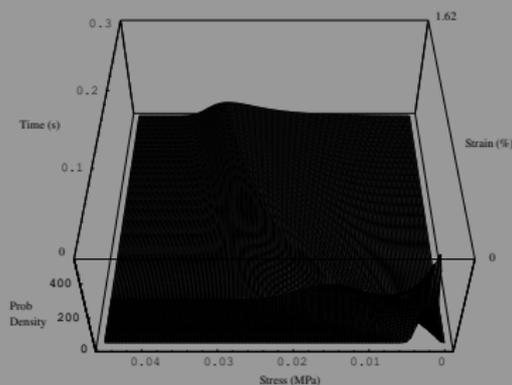
$$N_{(2)}^{(i)} = \int_0^t d\tau \text{cov} \left[\eta^{(i)}(t); \eta^{(i)}(t - \tau) \right]$$

Low OCR Cam Clay with Random G



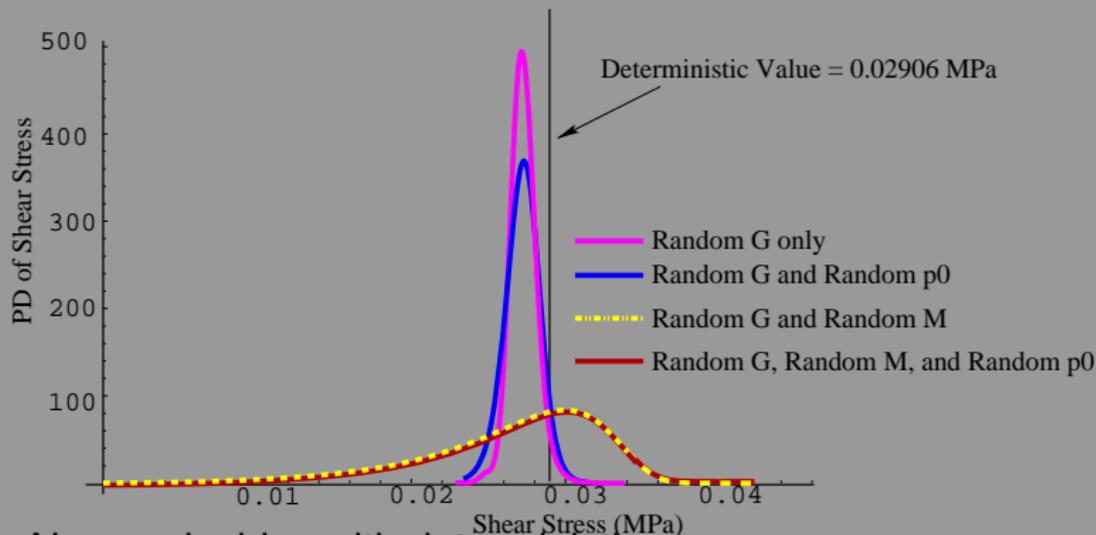
- ▶ Approximation of I.C.
- ▶ Non-symmetry in probability distribution!
- ▶ Response at critical state fairly certain but different than deterministic

Low OCR Cam Clay with Random G , M and ρ_0



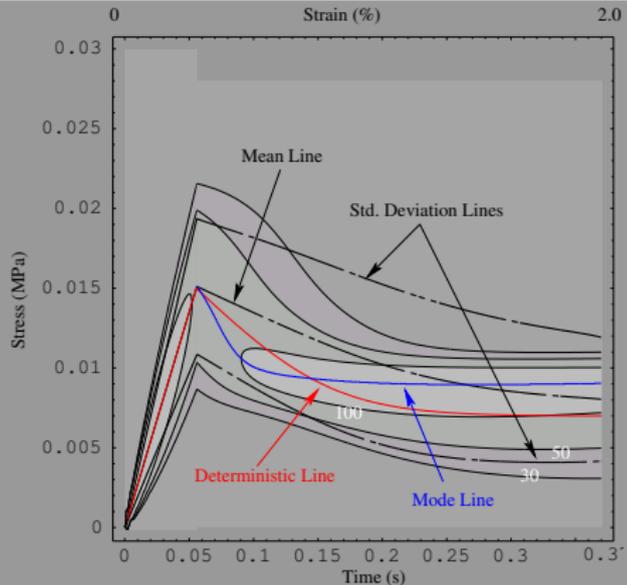
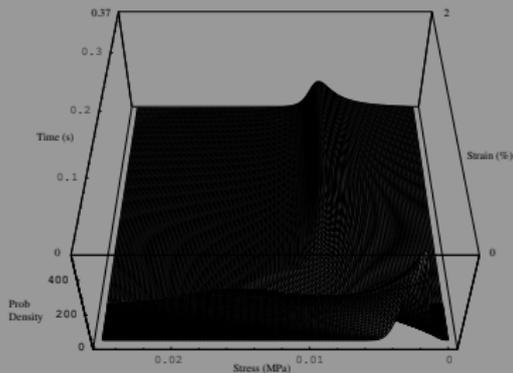
- ▶ Non-symmetry in probability distribution
- ▶ Difference between mean, mode and deterministic responses
- ▶ Divergence at critical state because M is uncertain

Comparison of Low OCR Cam Clay at $\epsilon = 1.62\%$



- ▶ None coincides with deterministic
- ▶ Some cases are very uncertain while some are fairly certain
- ▶ Either on safe or unsafe side

High OCR Cam Clay with Random G and M



- ▶ Very uncertain transition between el. & el.-pl.
- ▶ **Differences** between mean, mode, and deterministic responses
- ▶ Divergence at critical state, M is uncertain

Conclusions

- ▶ A new approach to account for uncertainties in elastic–plastic material simulation
- ▶ Methodology, which results in a FPK equation, overcomes the drawbacks of *Monte Carlo Method* and *Perturbation Technique*
- ▶ Advantage of FPK equation is evident as it transforms the original non–linear stochastic ODE to a linear deterministic PDE
- ▶ Developed methodology is capable of providing complete probabilistic description (PDF) of the solution
- ▶ Development is general in nature and applicable to any incremental elastic–plastic material model