

Решење еласто-пластичног проблема у простору вероватноћа и примена на практичне проблеме

Борис Јеремић,
Kallol Sett (UA), Lev Kavvas (UCD),
Сузана Копривица (УУ)

University of California, Davis

Универзитет УНИОН
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Outline

Motivation

- Stochastic Systems: Historical Perspectives
- Uncertainties in Material

Probabilistic Elasto–Plasticity

- PEP Formulations
- Probabilistic Elastic–Plastic Response

Stochastic Elastic–Plastic Finite Element Method

- SEPFEM Formulations
- SEPFEM Verification Example

Applications

- Seismic Wave Propagation Through Uncertain Soils
- Probabilistic Analysis for Decision Making

Закључак

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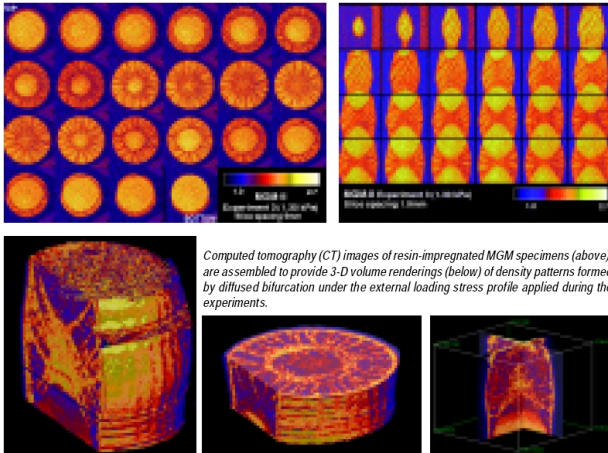
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Закључак

Failure Mechanisms for Geomaterials



Soil: Inside Failure of "Uniform" MGM Specimen

Personal Motivation

- ▶ Вероватноћа броја риба у језеру
- ▶ Williams' DEM simulations, differential displacement vortices
- ▶ SFEM round table
- ▶ Kavvas' probabilistic hydrology

Types of Uncertainties

- ▶ Epistemic uncertainty - due to lack of knowledge
 - ▶ Can be reduced by collecting more data
 - ▶ Mathematical tools are not well developed
 - ▶ trade-off with aleatory uncertainty
- ▶ Aleatory (случајне) uncertainty - inherent variation of physical system
 - ▶ Can not be reduced
 - ▶ Has highly developed mathematical tools



Ergodicity

- ▶ Exchange ensemble averages for time averages
- ▶ Is soil elasto-plasticity ergodic?
 - ▶ Can soil elastic–plastic statistical properties be obtained by temporal averaging?
 - ▶ Will soil elastic–plastic statistical properties "renew" at each occurrence?
 - ▶ Are soil elastic–plastic statistical properties statistically independent?
- ▶ Claim in literature that structural nonlinear behavior is non–ergodic while earthquake characteristics are (?!)
- ▶ However, earthquake characteristics is representing mechanics (fault slip) on a different scale...

Historical Overview

- ▶ Brownian motion, Langevin equation → PDF governed by simple diffusion Eq. (Einstein 1905)
- ▶ Approach for random forcing → relationship between the autocorrelation function and spectral density function (Wiener 1930)
- ▶ With external forces → Fokker-Planck-Kolmogorov (FPK) for the PDF (Kolmogorov 1941)
- ▶ Approach for random coefficient → Functional integration approach (Hopf 1952), Averaged equation approach (Bharrucha-Reid 1968), Numerical approaches, Monte Carlo method

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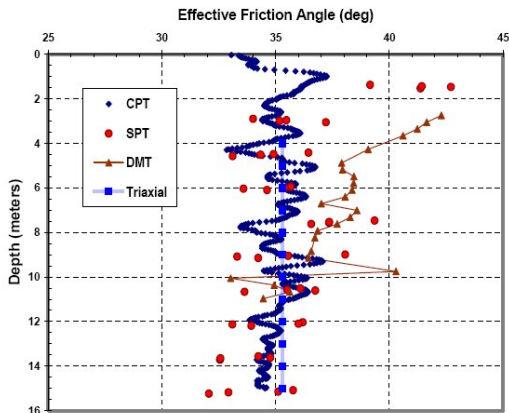
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Material Behavior Inherently Uncertain

- Spatial variability
- Point-wise uncertainty, testing error, transformation error



(Mayne et al. (2000))

Recent State-of-the-Art

- ▶ Governing equation
 - ▶ Dynamic problems $\rightarrow M\ddot{u} + C\dot{u} + Ku = F$
 - ▶ Static problems $\rightarrow Ku = F$
- ▶ Existing solution methods
 - ▶ **Random r.h.s** (external force random)
 - ▶ FPK equation approach
 - ▶ Use of fragility curves with deterministic FEM (DFEM)
 - ▶ **Random l.h.s** (material properties random)
 - ▶ Monte Carlo approach with DFEM $\rightarrow$ CPU expensive
 - ▶ Perturbation method $\rightarrow$ a linearized expansion! Error increases as a function of COV
 - ▶ Spectral method $\rightarrow$ developed for elastic materials so far
- ▶ New developments for elasto–plastic applications

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Uncertainty Propagation through Constitutive Eq.

- Incremental el–pl constitutive equation $\frac{d\sigma_{ij}}{dt} = D_{ijkl} \frac{d\epsilon_{kl}}{dt}$

$$D_{ijkl} = \begin{cases} D_{ijkl}^{el} & \text{for elastic} \\ D_{ijkl}^{el} - \frac{D_{ijmn}^{el} m_{mn} n_{pq} D_{pqkl}^{el}}{n_{rs} D_{rstu}^{el} m_{tu} - \xi_* r_*} & \text{for elastic–plastic} \end{cases}$$

Previous Work

- ▶ Linear algebraic or differential equations → Analytical solution:
 - ▶ Variable Transf. Method (Montgomery and Runger 2003)
 - ▶ Cumulant Expansion Method (Gardiner 2004)
- ▶ Nonlinear differential equations (elasto-plastic/viscoelastic-viscoplastic):
 - ▶ Monte Carlo Simulation (Schueller 1997, De Lima et al 2001, Mellah et al. 2000, Griffiths et al. 2005...)
 - accurate, very costly
 - ▶ Perturbation Method (Anders and Hori 2000, Kleiber and Hien 1992, Matthies et al. 1997)
 - first and second order Taylor series expansion about mean - limited to problems with small C.O.V. and inherits "closure problem"

Problem Statement

- Incremental 3D elastic-plastic stress–strain:

$$\frac{d\sigma_{ij}}{dt} = \left\{ D_{ijkl}^{el} - \frac{D_{ijmn}^{el} m_{mn} n_{pq} D_{pqkl}^{el}}{n_{rs} D_{rstu}^{el} m_{tu} - \xi_* r_*} \right\} \frac{d\epsilon_{kl}}{dt}$$

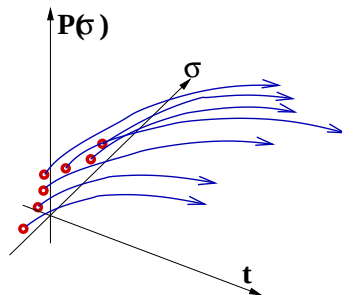
- Focus on 1D → a nonlinear ODE with random coefficient (material) and random forcing (ϵ)

$$\begin{aligned} \frac{d\sigma(x, t)}{dt} &= \beta(\sigma(x, t), D^{el}(x), q(x), r(x); x, t) \frac{d\epsilon(x, t)}{dt} \\ &= \eta(\sigma, D^{el}, q, r, \epsilon; x, t) \end{aligned}$$

with initial condition $\sigma(0) = \sigma_0$

Еволуција Густине Напона $\rho(\sigma, t)$

- From each initial point in σ -space a trajectory starts out describing the corresponding solution of the stochastic process
- Movement of a cloud of initial points described by density $\rho(\sigma, 0)$ in σ -space, is governed by the constitutive equation,



Stochastic Continuity (Liouville) Equation

- density ρ of $\sigma(x, t)$ varies in time according to a continuity Liouville equation (Kubo 1963):

$$\frac{\partial \rho(\sigma(x, t), t)}{\partial t} = - \frac{\partial \eta(\sigma(x, t), D^{el}(x), q(x), r(x), \epsilon(x, t))}{\partial \sigma} \rho[\sigma(x, t), t]$$

- with initial conditions $\rho(\sigma, 0) = \delta(\sigma - \sigma_0)$

Ensemble Average form of Liouville Equation

Continuity equation written in ensemble average form (eg. cumulant expansion method (Kavvas and Karakas 1996)):

$$\begin{aligned}
 \frac{\partial \langle \rho(\sigma(x_t, t), t) \rangle}{\partial t} = & - \frac{\partial}{\partial \sigma} \left[\left\{ \left\langle \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)) \right\rangle \right. \right. \\
 + & \int_0^t d\tau \text{Cov}_0 \left[\frac{\partial \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t))}{\partial \sigma}; \right. \\
 & \left. \left. \eta(\sigma(x_{t-\tau}, t - \tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t - \tau)) \right\rangle \right] \langle \rho(\sigma(x_t, t), t) \rangle \Big] \\
 + & \frac{\partial^2}{\partial \sigma^2} \left[\left\{ \int_0^t d\tau \text{Cov}_0 \left[\eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)); \right. \right. \right. \\
 & \left. \left. \left. \eta(\sigma(x_{t-\tau}, t - \tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t - \tau)) \right] \right\} \langle \rho(\sigma(x_t, t), t) \rangle \right]
 \end{aligned}$$

Eulerian–Lagrangian FPK Equation

van Kampen's Lemma (van Kampen 1976) $\rightarrow \langle \rho(\sigma, t) \rangle = P(\sigma, t)$,
ensemble average of phase density is the probability density;

$$\begin{aligned} \frac{\partial P(\sigma(x_t, t), t)}{\partial t} &= -\frac{\partial}{\partial \sigma} \left[\left\{ \left\langle \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)) \right\rangle \right. \right. \\ &+ \int_0^t d\tau \text{Cov}_0 \left[\frac{\partial \eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t))}{\partial \sigma}; \right. \\ &\quad \left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right] \right\} P(\sigma(x_t, t), t) \Big] \\ &+ \frac{\partial^2}{\partial \sigma^2} \left[\left\{ \int_0^t d\tau \text{Cov}_0 \left[\eta(\sigma(x_t, t), D^{el}(x_t), q(x_t), r(x_t), \epsilon(x_t, t)); \right. \right. \right. \\ &\quad \left. \left. \eta(\sigma(x_{t-\tau}, t-\tau), D^{el}(x_{t-\tau}), q(x_{t-\tau}), r(x_{t-\tau}), \epsilon(x_{t-\tau}, t-\tau)) \right] \right\} P(\sigma(x_t, t), t) \Big] \end{aligned}$$

Eulerian–Lagrangian FPK Equation

- Advection-diffusion equation

$$\frac{\partial P(\sigma, t)}{\partial t} = -\frac{\partial}{\partial \sigma} \left[N_{(1)} P(\sigma, t) - \frac{\partial}{\partial \sigma} \{ N_{(2)} P(\sigma, t) \} \right]$$

- Complete probabilistic description of response
- Solution PDF is second-order exact to covariance of time (exact mean and variance)
- It is deterministic equation in probability density space
- It is linear PDE in probability density space → simplifies the numerical solution process

Template Solution of FPK Equation

- FPK diffusion–advection equation is applicable to any material model → only the coefficients $N_{(1)}$ and $N_{(2)}$ are different for different material models

$$\frac{\partial P(\sigma, t)}{\partial t} = -\frac{\partial}{\partial \sigma} \left[N_{(1)} P(\sigma, t) - \frac{\partial}{\partial \sigma} \{ N_{(2)} P(\sigma, t) \} \right] = -\frac{\partial \zeta}{\partial \sigma}$$

- Initial condition
 - Deterministic → Dirac delta function → $P(\sigma, 0) = \delta(\sigma)$
 - Random → Any given distribution
- Boundary condition: Reflecting BC → conserves probability mass $\zeta(\sigma, t)|_{At \text{ Boundaries}} = 0$
- Finite Differences used for solution (among many others)

Application of FPK equation to Material Models

- ▶ FPK equation is applicable to any incremental elastic–plastic material model
- ▶ Solution in terms of PDF, not a single value of stress
- ▶ Influence of initial condition on the PDF of stress
- ▶ Mean stress yielding or
- ▶ Probabilistic yielding

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Закључак

Еластичан материјал са случајним модулом еластичности G

- General form of elastic constitutive rate equation

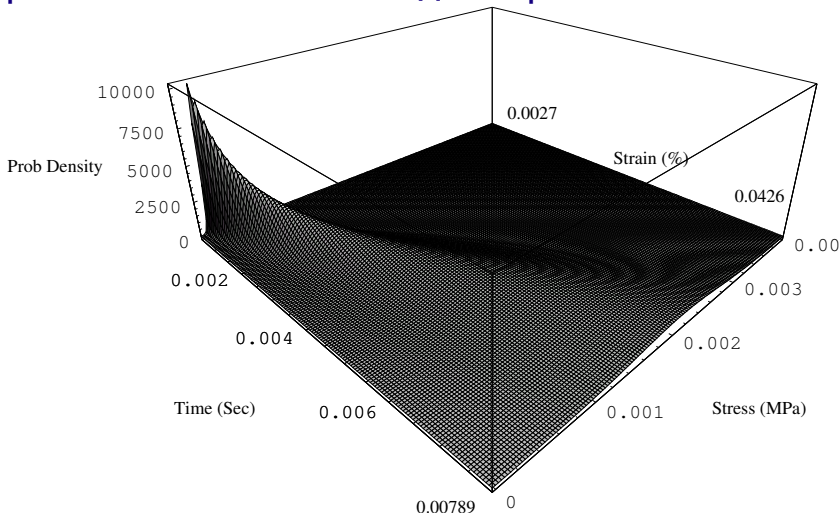
$$\frac{d\sigma_{12}}{dt} = 2G \frac{d\epsilon_{12}}{dt} = \eta(G, \epsilon_{12}; t)$$

- Advection and diffusion coefficients of FPK equation

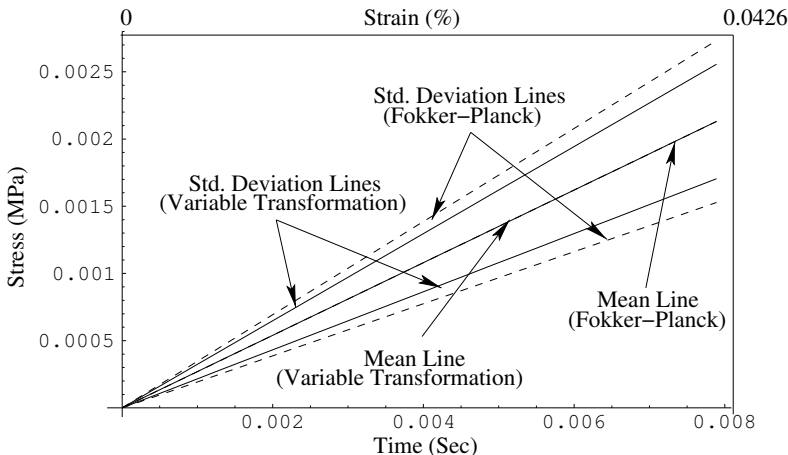
$$N_{(1)} = 2 \frac{d\epsilon_{12}}{dt} \langle G \rangle \quad ; \quad N_{(2)} = 4t \left(\frac{d\epsilon_{12}}{dt} \right)^2 \text{Var}[G]$$

- Пример: $\langle G \rangle = 2.5 \text{ MPa}$; Std. Deviation $[G] = 0.5 \text{ MPa}$

Вероватан еластичан одговор



Верификација – Variable Transformation Method



Drucker-Prager Linear Hardening with Random G

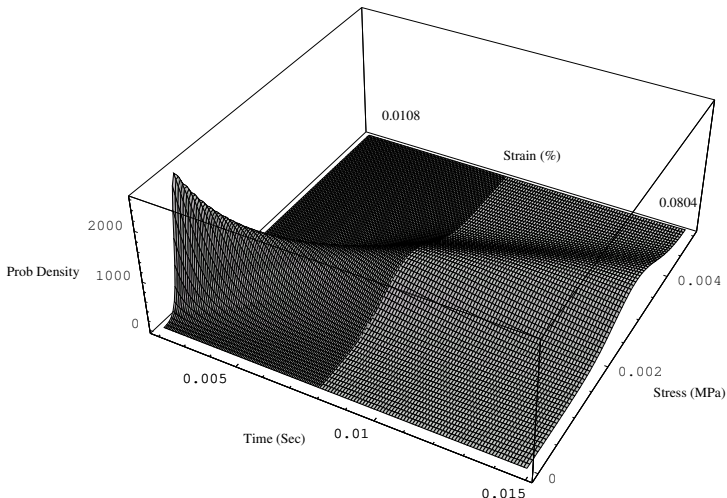
$$\frac{d\sigma_{12}}{dt} = G^{ep} \frac{d\epsilon_{12}}{dt} = \eta(\sigma_{12}, G, K, \alpha, \alpha', \epsilon_{12}; t)$$

Advection and diffusion coefficients of FPK equation

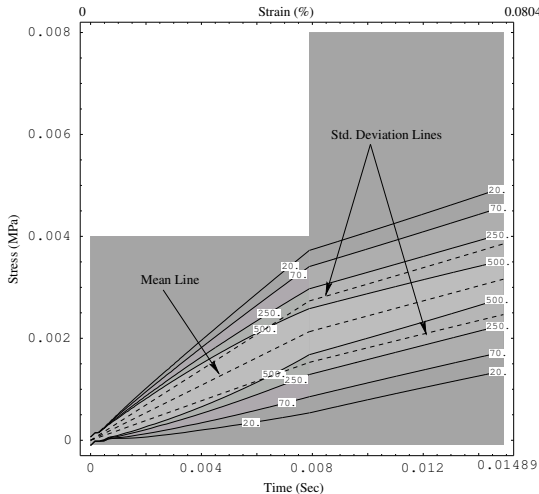
$$N_{(1)} = \frac{d\epsilon_{12}}{dt} \left\langle 2G - \frac{G^2}{G + 9K\alpha^2 + \frac{1}{\sqrt{3}}l_1\alpha'} \right\rangle$$

$$N_{(2)} = t \left(\frac{d\epsilon_{12}}{dt} \right)^2 \text{Var} \left[2G - \frac{G^2}{G + 9K\alpha^2 + \frac{1}{\sqrt{3}}l_1\alpha'} \right]$$

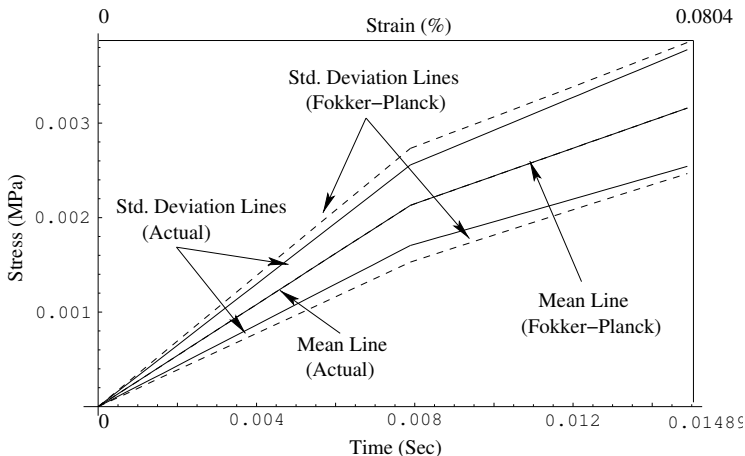
Drucker-Prager Linear Hardening, Random G



Drucker-Prager Linear Hardening, Random G



Verification of D–P E–P Response - Monte Carlo



Modified Cam Clay Constitutive Model

$$\frac{d\sigma_{12}}{dt} = G^{ep} \frac{d\epsilon_{12}}{dt} = \eta(\sigma_{12}, G, M, e_0, p_0, \lambda, \kappa, \epsilon_{12}; t)$$

$$\eta = \left[2G - \frac{\left(36 \frac{G^2}{M^4} \right) \sigma_{12}^2}{\frac{(1 + e_0)p(2p - p_0)^2}{\kappa} + \left(18 \frac{G}{M^4} \right) \sigma_{12}^2 + \frac{1 + e_0}{\lambda - \kappa} p p_0 (2p - p_0)} \right]$$

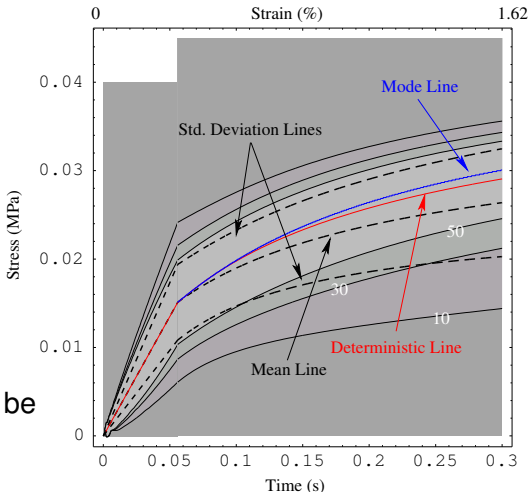
Advection and diffusion coefficients of FPK equation

$$N_{(1)}^{(i)} = \langle \eta^{(i)}(t) \rangle + \int_0^t d\tau \text{cov} \left[\frac{\partial \eta^{(i)}(t)}{\partial t}; \eta^{(i)}(t - \tau) \right]$$

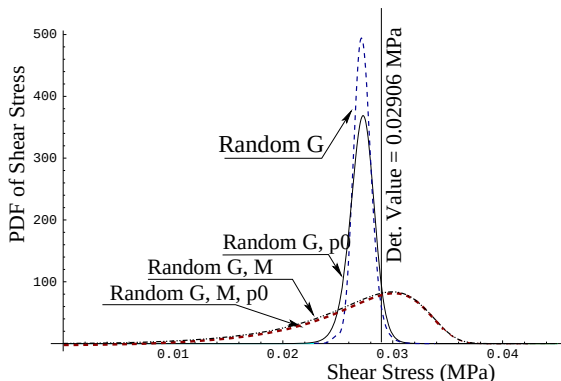
$$N_{(2)}^{(i)} = \int_0^t d\tau \text{cov} \left[\eta^{(i)}(t); \eta^{(i)}(t - \tau) \right]$$

Low OCR Cam Clay with Random G , M and p_0

- ▶ Non-symmetry in probability distribution
- ▶ Difference between mean, mode and deterministic
- ▶ Divergence at critical state because M might be (is) uncertain?



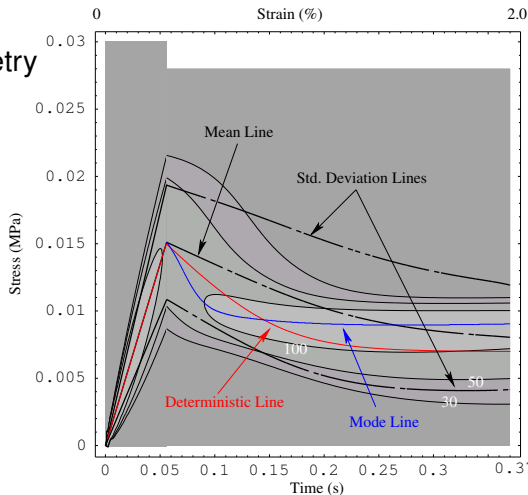
Comparison of Low OCR Cam Clay at $\epsilon = 1.62\%$



- ▶ None coincides with deterministic
- ▶ Some very uncertain, some very certain
- ▶ Either on safe or unsafe side

High OCR Cam Clay with Random G and M

- ▶ Large non-symmetry in probability distribution
- ▶ Significant differences in mean, mode, and deterministic
- ▶ Divergence at critical state, M is uncertain



Probabilistic Yielding

- ▶ Weighted elastic and elastic–plastic solution

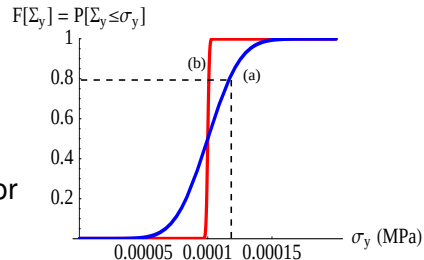
$$\partial P(\sigma, t) / \partial t = -\partial \left(N_{(1)}^w P(\sigma, t) - \partial \left(N_{(2)}^w P(\sigma, t) \right) / \partial \sigma \right) / \partial \sigma$$

- ▶ Weighted advection and diffusion coefficients are then

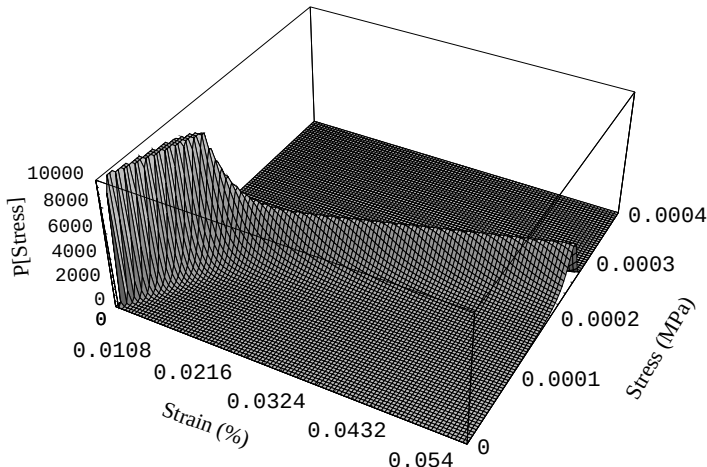
$$N_{(1,2)}^w(\sigma) = (1 - P[\Sigma_y \leq \sigma]) N_{(1)}^{el} + P[\Sigma_y \leq \sigma] N_{(1)}^{el-pl}$$

- ▶ Cumulative Density Function (CDF) of the yield function

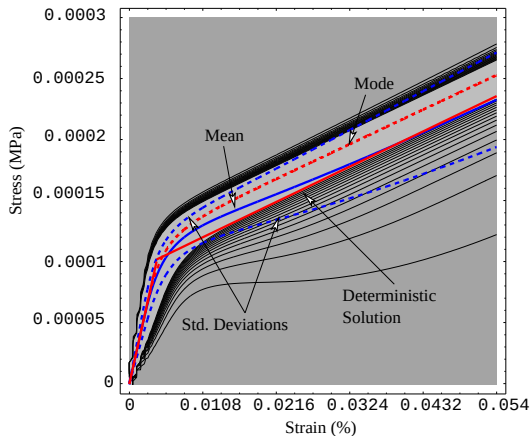
- ▶ Similar to European Pricing Option in financial simulations (Black–Scholes options pricing model '73, Nobel prize for Economics '97)



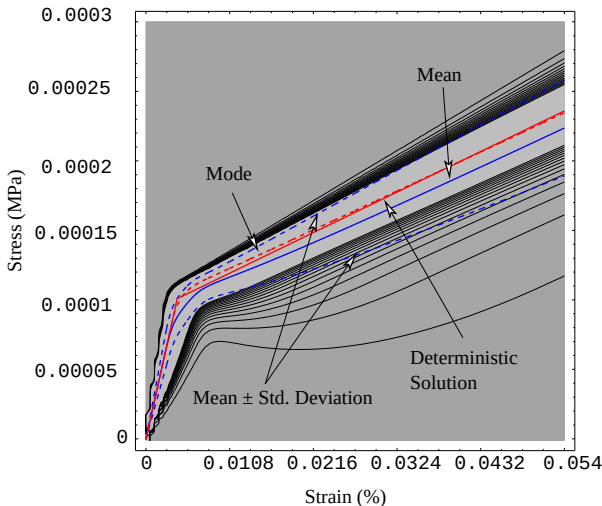
Bi-Linear von Mises Response



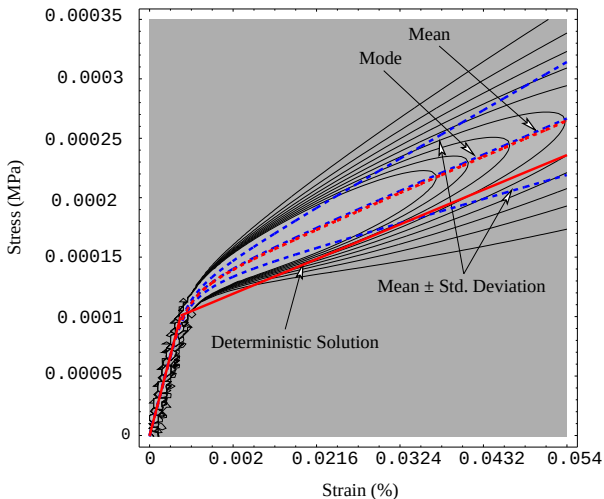
Bi-Linear von Mises Response



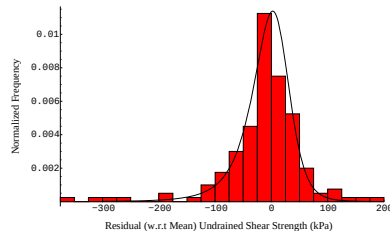
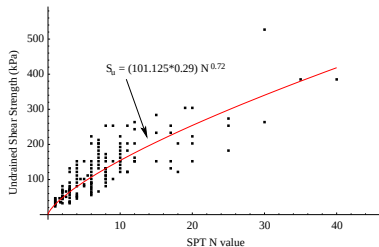
Drucker Prager: Uncertain G , Certain ϕ



Drucker Prager: Certain G , Uncertain ϕ

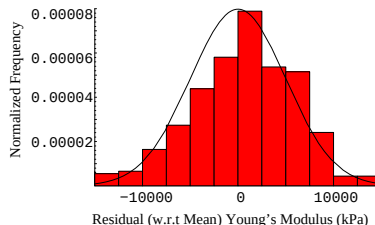
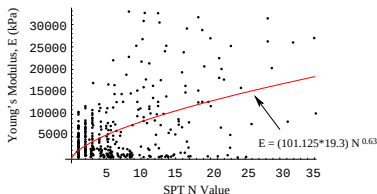


SPT Based Determination of Shear Strength



Transformation of SPT N -value $\rightarrow$ undrained shear strength, s_u
 (cf. Phoon and Kulhawy (1999B))
 Histogram of the residual (w.r.t the deterministic transformation equation) undrained strength, along with fitted probability density function (Pearson IV)

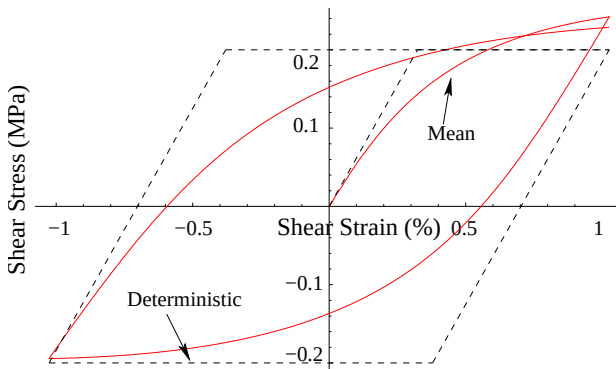
SPT Based Determination of Young's Modulus



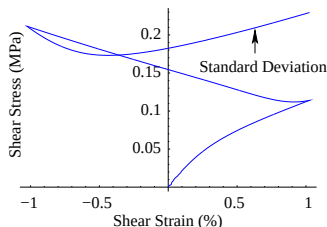
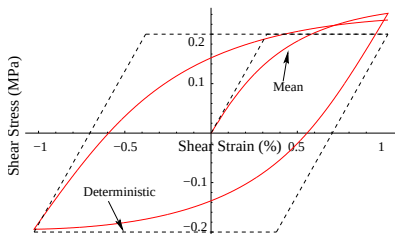
Transformation of SPT N -value $\rightarrow$ 1-D Young's modulus, E (cf. Phoon and Kulhawy (1999B))

Histogram of the residual (w.r.t the deterministic transformation equation) Young's modulus, along with fitted probability density function

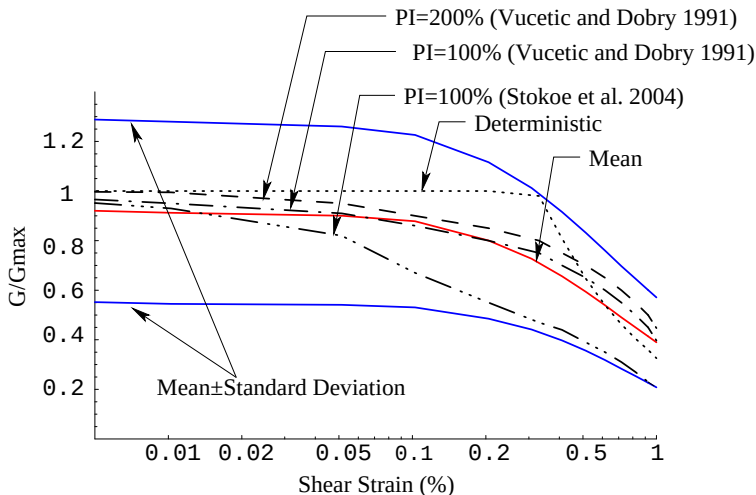
Вероватан одговор материјала (von–Mises)



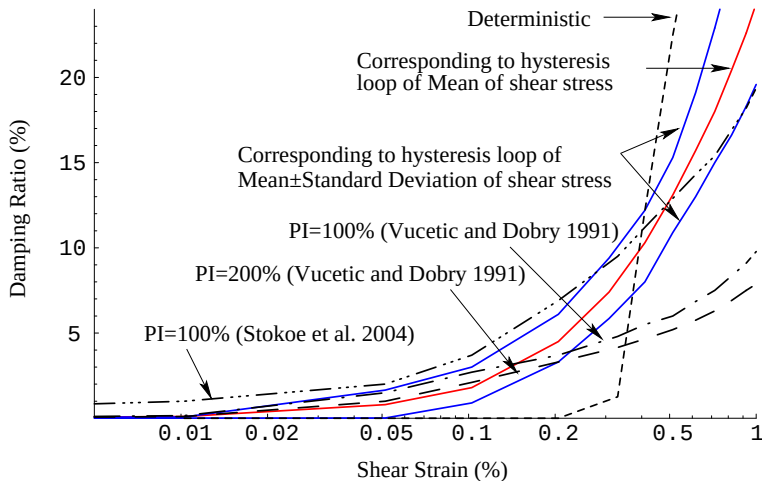
Вероватан одговор материјала уз стандардну девијацију



G/G_{max} Response



Damping Response



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Stochastic Finite Element Formulation

- Governing equations:

$$A\sigma = \phi(t); \quad Bu = \epsilon; \quad \sigma = D\epsilon$$

- **Spatial** and **stochastic** discretization

- Deterministic spatial differential operators (A & B) → Regular shape function method with Galerkin scheme
- Input random field material properties (D) → Karhunen–Loève (KL) expansion, optimal expansion, error minimizing property
- Unknown solution random field (u) → Polynomial Chaos (PC) expansion

Spectral Stochastic Elastic–Plastic FEM

- Minimizing norm of error of finite representation using Galerkin technique (Ghanem and Spanos 2003):

$$\sum_{n=1}^N K_{mn}^{ep} d_{ni} + \sum_{n=1}^N \sum_{j=0}^P d_{nj} \sum_{k=1}^M C_{ijk} K_{mnk}'^{ep} = \langle F_m \psi_i[\{\xi_r\}] \rangle$$

$$K_{mn}^{ep} = \int_D B_n \mathbf{D}^{ep} B_m dV$$

$$C_{ijk} = \langle \xi_k(\theta) \psi_i[\{\xi_r\}] \psi_j[\{\xi_r\}] \rangle$$

$$K_{mnk}'^{ep} = \int_D B_n \sqrt{\lambda_k} h_k B_m dV$$

$$F_m = \int_D \phi N_m dV$$

Inside SEPFEM

- ▶ Explicit stochastic elastic–plastic finite element computations
- ▶ FPK probabilistic constitutive integration at Gauss integration points
- ▶ Increase in (stochastic) dimensions (KL and PC) of the problem (parallelism)
- ▶ Development of the probabilistic elastic–plastic stiffness tensor

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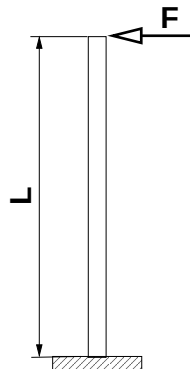
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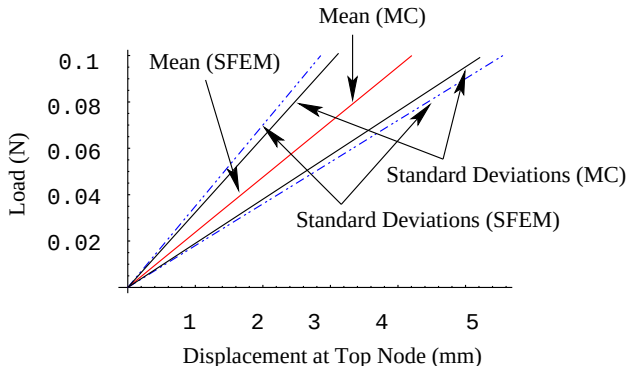
Закључак

1–D Static Pushover Test Example

- ▶ Linear elastic model:
 $\langle G \rangle = 2.5 \text{ kPa}$,
 $\text{Var}[G] = 0.15 \text{ kPa}^2$,
 correlation length for $G = 0.3 \text{ m}$.
- ▶ Elastic–plastic material model,
 von Mises, linear hardening,
 $\langle G \rangle = 2.5 \text{ kPa}$,
 $\text{Var}[G] = 0.15 \text{ kPa}^2$,
 correlation length for $G = 0.3 \text{ m}$,
 $C_u = 5 \text{ kPa}$,
 $C'_u = 2 \text{ kPa}$.

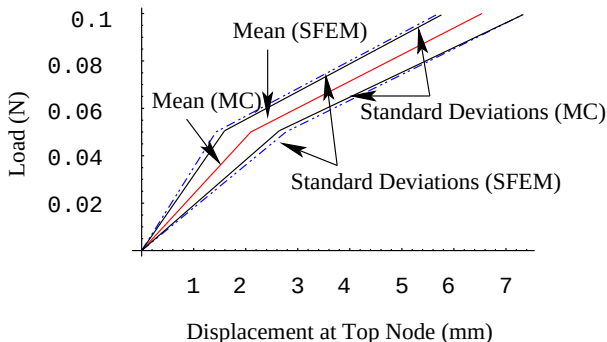


Linear Elastic FEM Verification



Mean and standard deviations of displacement at the top node,
linear elastic material model,
KL-dimension=2, order of PC=2.

SEPFEM verification



Mean and standard deviations of displacement at the top node, von Mises elastic-plastic linear hardening material model, KL-dimension=2, order of PC=2.

Outline

Motivation

Stochastic Systems: Historical Perspectives
Uncertainties in Material

Probabilistic Elasto–Plasticity

PEP Formulations
Probabilistic Elastic–Plastic Response

Stochastic Elastic–Plastic Finite Element Method

SEPFEM Formulations
SEPFEM Verification Example

Applications

Seismic Wave Propagation Through Uncertain Soils
Probabilistic Analysis for Decision Making

Закључак

Applications

- ▶ Stochastic elastic–plastic simulations of soils and structures
- ▶ Probabilistic inverse problems
- ▶ Geotechnical site characterization design
- ▶ Optimal material design

Seismic Wave Propagation through Stochastic Soil

- ▶ Soil as 12.5 m deep 1–D soil column (von Mises Material)
 - ▶ Properties (including testing uncertainty) obtained through random field modeling of CPT q_T

$$\langle q_T \rangle = 4.99 \text{ MPa}; \quad \text{Var}[q_T] = 25.67 \text{ MPa}^2;$$

$$\text{Cor. Length } [q_T] = 0.61 \text{ m}; \quad \text{Testing Error} = 2.78 \text{ MPa}^2$$
- ▶ q_T was transformed to obtain G : $G/(1 - \nu) = 2.9q_T$
 - ▶ Assumed transformation uncertainty = 5%

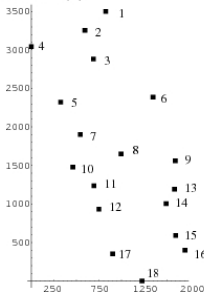
$$\langle G \rangle = 11.57 \text{ MPa}; \quad \text{Var}[G] = 142.32 \text{ MPa}^2$$

$$\text{Cor. Length } [G] = 0.61 \text{ m}$$
- ▶ Input motions: modified 1938 Imperial Valley

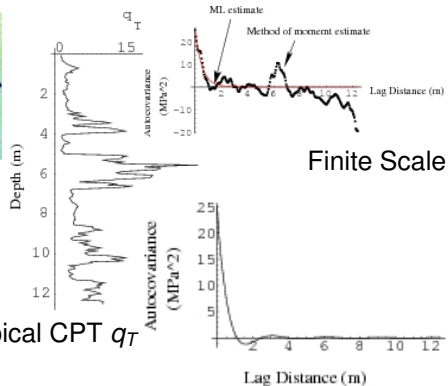
Random Field Parameters from Site Data

► Maximum likelihood estimates

S-N Coordinate (m)



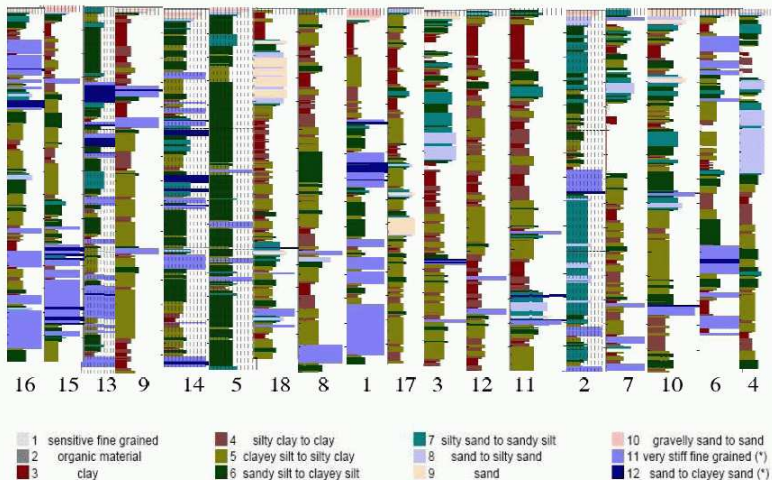
W-E Coordinate (m)



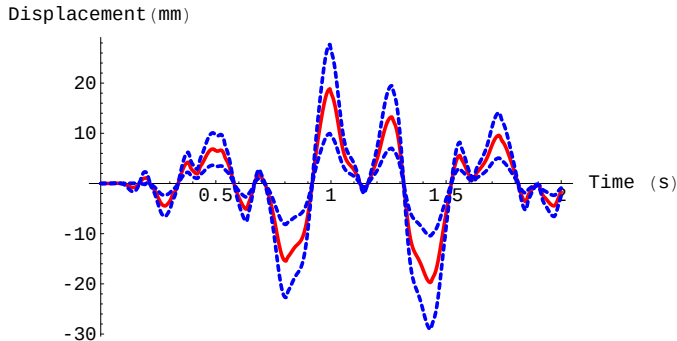
Finite Scale

Fractal

"Uniform" CPT Site Data



Seismic Wave Propagation through Stochastic Soil



Mean $\pm$ Standard Deviation

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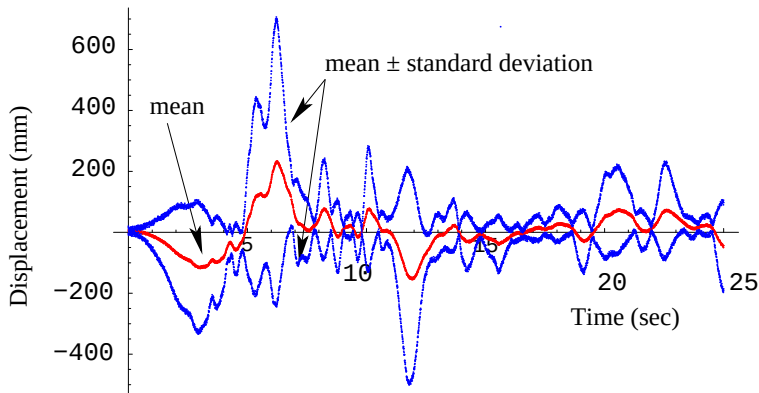
Seismic Wave Propagation Through Uncertain Soils
Probabilistic Analysis for Decision Making

Закључак

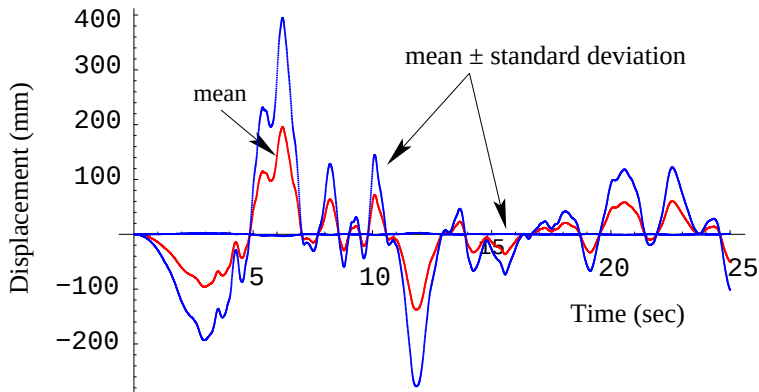
Approaches to Modeling

- ▶ Do nothing about site characterization (rely on experience): conservative **guess** of soil data, $COV = 225\%$, correlation length = 12m.
- ▶ Do better than standard site characterization: $COV = 103\%$, correlation length = 0.61m)
- ▶ Improve site characterization if probabilities of exceedance are unacceptable!

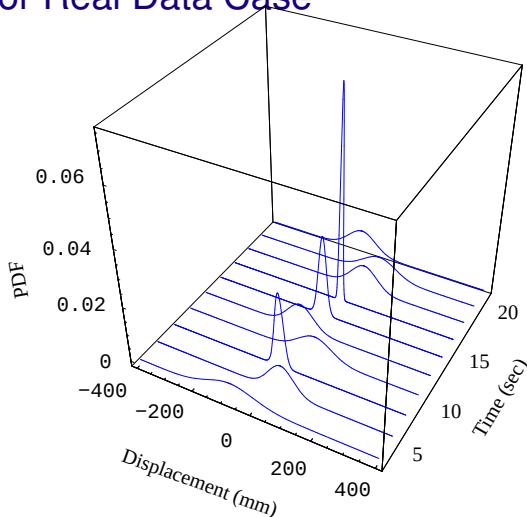
Evolution of Mean $\pm$ SD for Guess Case



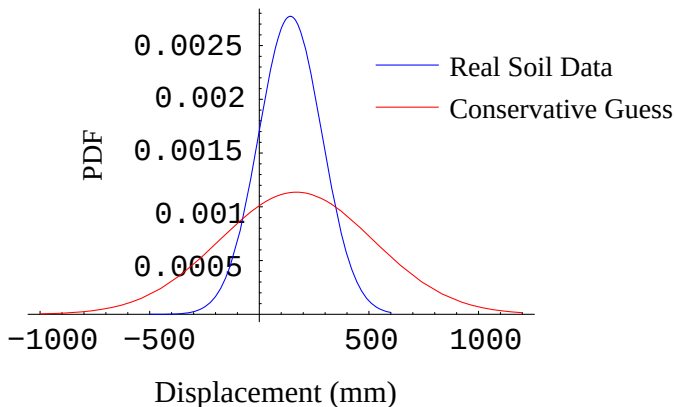
Evolution of Mean $\pm$ SD for Real Data Case



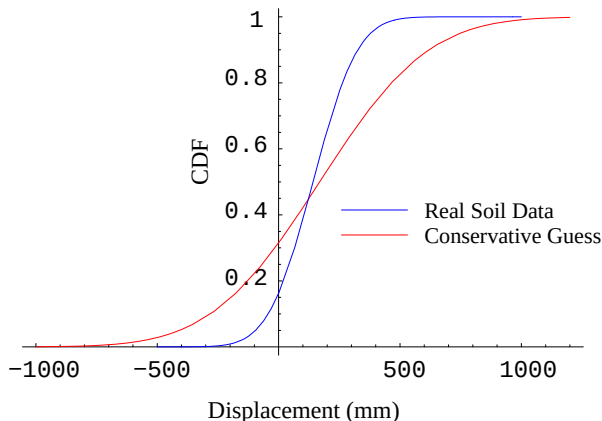
Full PDFs for Real Data Case



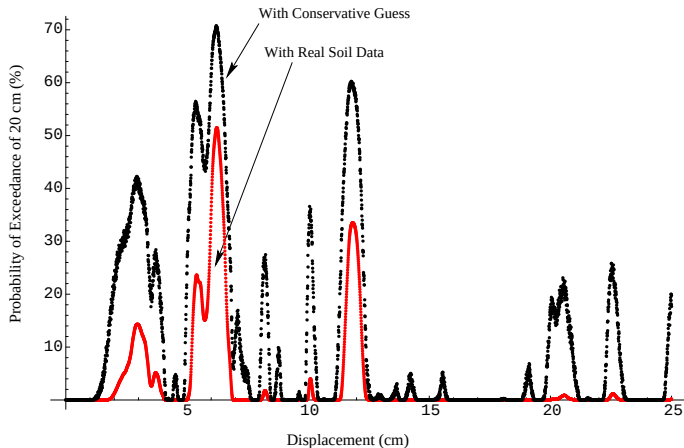
Example: PDF at 6 s



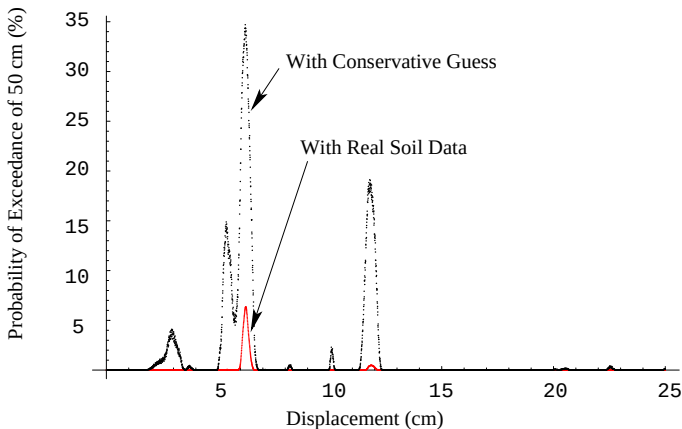
Example: CDF at 6 s



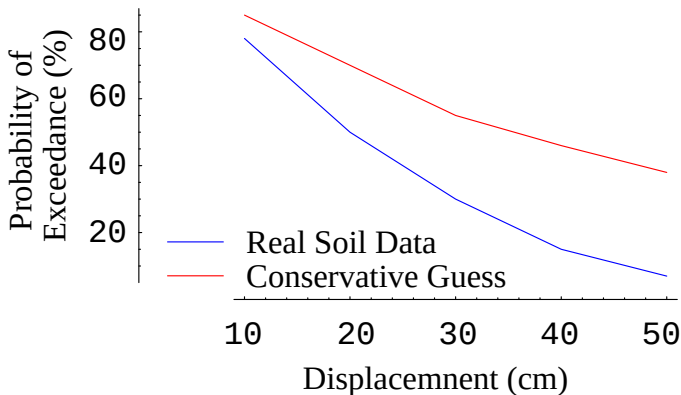
Probability of Exceedance of 20cm



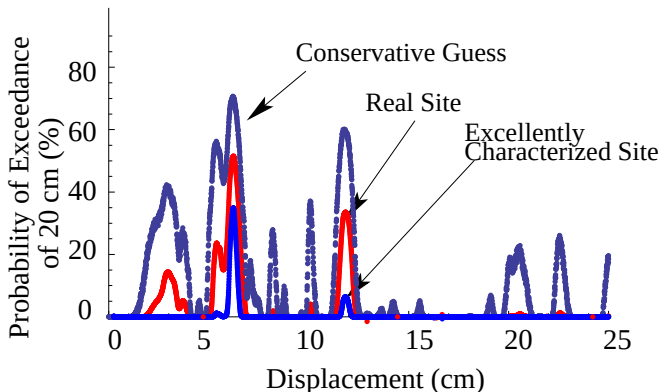
Probability of Exceedance of 50cm



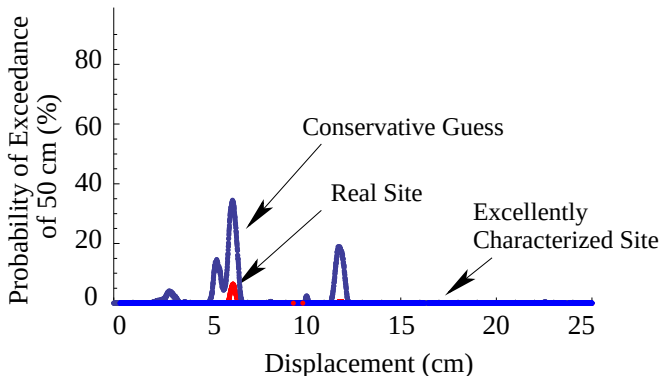
Probabilities of Exceedance vs. Displacements



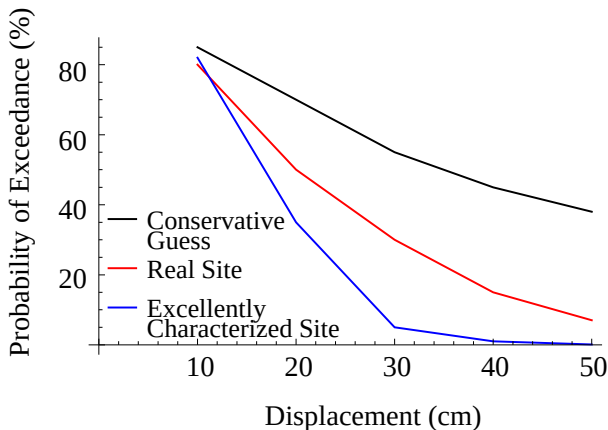
Шта ако имамо још бољу карактеризацију материјала



Вероватноћа недопустивих деформација се може још оборити



Вероватно прихватљиве деформације



Закључак

- ▶ Понашање материјала се вероватно најбоље може описати теоријом вероватноћа
- ▶ Прорачуне вероватно треба радити користећи теорију вероватноћа (пре него што нас банке и осигуравајућа друштва на то натерају)
- ▶ Научне и инжењерске методе за такве прорачуне већ постоје (тачности другог реда) и и даље се усавршавају
- ▶ Могућ проблем је људска природа: колико желите да знате о (не–) вероватним проблемима?